APRIL 2021 ENERGINET ELTRANSMISSION A/S

THOR OFFSHORE WIND FARM LANDFALL SITE INVESTIGATIONS

GEOTECHNICAL INVESTIGATION REPORT NO. 1





ADDRESS COWI A/S Nupark 51 7500 Holstebro Denmark

> TEL +45 56 40 00 00 FAX +45 56 40 99 99 WWW cowi.com

APRIL 2021 ENERGINET ELTRANSMISSION A/S

THOR OFFSHORE WIND FARM LANDFALL SITE INVESTIGATIONS

GEOTECHNICAL INVESTIGATION REPORT NO. 1

PROJECT NO. DOCUMENT NO. A224691 A224691-001.1 VERSION DATE OF ISSUE DESCRIPTION PREPARED CHECKED APPROVED 2.0 21 April 2021 Three Prepared Prepare

CONTENTS

1	Purpose of the investigation	6
2	Project	6
3	Field investigation and laboratory work	6
4	Levels and coordinates	8
5 5.1 5.2 5.3 5.4	Subsurface conditions General Geology Particle Size Distribution Thermal Conductivity	9 9 10 11 12
6	Water levels	12
7	Geotechnical interpretations	13
8	Final remarks	13

ENCLOSURES

Legend for Borehole Logs	A-1
Borehole and CPT Logs	
Borehole Log, B/CPT 1	1.1
CPT Log, CPT 1	1.1CPT
CPT Log, CPT 2	1.2CPT
CPT Log, CPT 3	1.3CPT
Borehole Log, B/CPT 4	1.4
CPT Log, CPT 4	1.4CPT
Borehole Log, B/CPT 5	1.5
CPT Log, CPT 5	1.5CPT
CPT Log, CPT 6	1.6CPT

Particle Size Distribution:

Particle Size Distribution, B/CPT 1, sample 30A and 40A	1.7
Particle Size Distribution, B/CPT 4, sample 22A and 43A	1.8
Particle Size Distribution, B/CPT 5, sample 26A and 44A	1.9

Thermal Conductivity

Thermal Conductivity, B/CPT 1, sample 40A	1.10
Thermal Conductivity, B/CPT 4, sample 43A	1.11
Thermal Conductivity, B/CPT 5, sample 26A	1.12

Profiles and plans

Geological Profile	1.13
Site Location Plan	1.14

1 Purpose of the investigation

A geotechnical investigation has been undertaken at the landfall site of the Thor Offshore Wind Farm project at Fjaltring.

The purpose of the investigation is to provide geological and geotechnical data in sufficient detail for the planning and design of the HDD at the site.

2 Project

The location of the landfall site and a planned 300m wide corridor for the HDD is shown on the location plan "G002", dated 10 August 2017 and prepared by Energinet Eltransmission A/S.

The location plan Encl. No. 1.14. is based on location plan "G002" and shows the planned centreline of corridor for the HDD.

Currently, there is no information about the planned depth of the HDD. However, the planed investigation depth for geotechnical investigation has been set to level -25m, DVR90 by Energinet.

3 Field investigation and laboratory work

The geotechnical field investigations were performed during the period 3 to 16 March 2021 and comprised 3 nos. geotechnical boreholes denoted B/CPT 1, B/CPT 3 and B/CPT 4, and 6 nos. CPTs denoted CPT 1 – CPT 6.

The locations of the boreholes and CPTs were surveyed using GPS equipment.

The locations of the boreholes and CPTs are shown on the Site Location Plan, Encl. No. 1.14.

The geotechnical boreholes were performed to a depth of 27.0 - 31.0 m below ground level (m b. g. l.).

The boreholes were drilled with a hydraulic rig, using casing, shell and auger techniques and with a borehole diameter of 150 mm.

The CPTs were performed to a depth of 12.8 – 21.5m b. g. l.

The depth of the CPTs were planned to match the depth of the neighbouring boreholes. However, all CPTs were terminated at maximum thrust of the CPT equipment at approx. 10 tons. Under level of refusal, SPT tests were performed in cohesionless soils for every 2 - 3m in the neighbouring borehole.

The CPTs were carried out with a piezocone, which enables measurements of tip and sleeve resistance as well as pore water pressure.

SPT tests shows the number of blows, N, necessary to achieve a penetration of the sampler of 300mm, $N_{\rm 300.}$

During the drilling, the thicknesses and depths of all encountered strata were noted. Representative, disturbed samples were recovered from all layers and at intervals not exceeding 0.5m.

In the boreholes undisturbed tube samples (A-tubes) or large, disturbed samples (approx. 10 kg) were recovered for approx. every 5.0m from approx. 5.0m b. g. l. A-tubes were recovered in cohesive soils and disturbed samples were recovered in sand deposits.

Field vane tests were carried out in cohesive soils in order to determine the undrained shear strengths of the materials in the undisturbed state, as well as after remoulding (c_{fv} respectively c_{fvr}).

A standpipe, consisting of Ø25mm PVC tube with slots at the base, was installed in the boreholes B/CPT 4 and B/CPT 5 to allow monitoring of the ground water level in the boreholes. A bentonite plug was installed above the filter to prevent surface water from influencing the water level in the standpipe.

Monitoring of the water levels was performed on 29 March 2021.

A geological description and classification of all soil samples was carried out in the laboratory. This work was performed in accordance with the guidelines in the Danish Geological Society's publication "Vejledning i ingeniørgeologisk prøvebeskrivelse" dated February 2009.

The carbonate content of the soil samples was qualitatively evaluated by the reaction of the samples to 10 % hydrocloric acid.

The natural water content, w, of the soil samples was determined on selected samples.

The bulk density of the soil was conducted on all suitable undisturbed samples.

The Plasticity Index by Atterberg limit tests was determined on selected soil samples.

The results of the field and laboratory work are presented on the borehole logs, Enclosure Nos. 1.1, 1.4, 1.5 and the CPT logs, Enclosure Nos. 1.1CPT – 1.6CPT.

The results of the above has been reported to the Jupiter Database.

8

The particle size distribution by sieve analysis and by combined sieve and hydrometer analysis was determined on selected soil samples. The results of these teste are given in table 3 and in Enclosure Nos. 1.7 - 1.9.

The thermal conductivity was determined on selected soil samples. The results of these teste are given in table 4 and in Enclosure Nos. 1.10 - 1.12.

A legend for the symbols used on the boreholes and CPT logs is presented in Encl. A-1.

4 Levels and coordinates

All depths are measured from the ground surface as meters below ground level, (m b. g. l.).

All levels refer to the Danish National Survey System, "Dansk Vertikal Reference 1990" (DVR90).

The system DVR90 is approximately coinciding with Mean Sea Level at the shoreline.

The coordinate system used for determining coordinates for the locations of the boreholes and CPTs is UTM32.

Levels and coordinates for all boreholes and CPTs are shown in Table 1.

Borehole No. CPT No.	Ground Level Elevation (m)	X (m)	Y (m)
B/CPT 1 CPT 1	+1.3	446,032	6,257,440
CPT 2	+1.2	446,012	6,257,295
CPT 3	+3.7	446,239	6,257,460
B/CPT 4 CPT 4	+2.7	446,221	6,257,293
B/CPT 5 CPT 5	+5.1	446,407	6,257,482
CPT 6	+3.3	446,366	6,257,310

Table 1:Levels and coordinates at boreholes and CPTs

5 Subsurface conditions

5.1 General

In general, the subsurface conditions encountered in the boreholes comprise of postglacial sand and gravel overlying late glacial and glacial clay, sand and gravel deposits. These deposits are then primarily overlying Miocene clay and sand deposits along with Miocene Gyttja.

In the boreholes B/CPT 4 and B/CPT 5 the upper 0.6 and 0.8 m comprises of topsoil.

Below the topsoil and from terrain in borehole B/CPT 1 postglacial sand and gravel is found to a depth of 3.7 - 4.9 m b. g. l. The postglacial deposits are underlain by late glacial and glacial sand, gravel and clay to 14.7 - 17.2 m b. g. l. Most of the clay deposits are very high plasticity clay.

In borehole B/CPT 5 the late glacial and glacial deposits are underlain by 0.3 m of glacial clay till. Under the clay till in borehole B/CPT 5, and under the late glacial and glacial deposits in the other boreholes, Miocene deposits, primarily consisting of clay and sand, are encountered to the base of the boreholes 27.0 - 30.0 m b. g. l. However, in borehole B/CPT 4 and B/CPT 5 Miocene gyttja is encountered from 22.5 to 25.3 m b. g. l. Most of the clay deposits are very high plasticity clay.

CPT tip resistances from 5 to >20 MPa are recorded in the late glacial sand, indicating that these deposits are medium dense to dense.

In the Miocene sand deposits, SPT tests results are generally greater than 50 blows pr. 300 mm, indicating that these deposits are generally very dense.

The subsurface conditions can be divided into the general layers as shown in Table 2.

Soil type	B/CPT 1 Elev. (m)	B/CPT 4 Elev. (m)	B/CPT 5 Elev. (m)
TOPSOIL and SAND, Re	-	+2.7 - +1.9	+5.1 - +4.5
SAND and GRAVEL, Pg	+1.33.3	+1.92.3	+4.5 - +1.4
SAND and GRAVEL, Lg	-	-2.32.5	+1.41.3
CLAY, Lg/Gc	-3.312.3	-2.59.4	-1.38.9
SAND, Lg/Gc	-	-9.411.3	-
CLAY, Lg/Gc	-	-11.312.3	-
GRAVEL, Lg/Gc	-	-12.312.5	-8.910.0
SAND, Lg/Gc	-12.313.4	-	-10.012.1
CLAY TILL, Gc	-	-	-12.112.4
CLAY, Mi	-	-12.513.6	-
SAND, Mi	-13.418.0	-13.619.8	-12.418.7
GYTTJA, Mi	-	-19.820.5	-18.720.2
CLAY, Mi	-18.022.8	-20.522.7	-20.225.9
SAND, Mi	-22.825.7	-22.725.3	-

Table 2:Subsurface conditions

5.2 Geology

The geological conditions that apply to the area around the planned line for the HDD are briefly described here. The description is based on data from the GeoAtlas and Jupiter database and boreholes and CPTs carried out in connection with the present project.

A conceptual geological profile along the expected corridor for the project can be seen in Enclosure Nos. 1.13.

As described in section 5.1 the area is generally made up of the following layers:

Postglacial SAND and GRAVEL (S1) overlying late glacial and glacial CLAY (C1), SAND and GRAVEL (S2) and CLAY TILL (C 2) deposits. These deposits are then primeryly overlying Miocen CLAY (C3) and SAND (S3) deposits along with Miocene GYTTJA (G1).

5.3 Particle Size Distribution

The particle size distribution by sieve analysis and by combined sieve and hydrometer analysis was determined on sample 30A and 40A from borehole B/CPT 1, sample 22A and 43A from borehole B/CPT 4 and sample 26A and 44A from borehole B/CPT 5. The results of these teste are given in Table 3 and in Enclosure Nos. 1.7 - 1.9.

Borehole No.	Sample No.	Geological Description	d₁₀ (mm)	d₅₀ (mm)	U (d ₆₀ /d ₁₀)
B/CPT 1	30A	SAND, fine, silty, micaceous, with small lignite fragments, sorted	0.07	0.16	2,7
B/CPT 1	40A	CLAY, very high plasticity, micaceous	_*	0.01	_*
B/CPT 4	22A	CLAY, very high plasticity	_*	0.01	_*
B/CPT 4	43A	SAND, fine, micaceous, with small fragments of lignite, sorted	0.06	0.11	2.1
B/CPT 5	26A	CLAY, very high plasticity, with many thin layers of silt	_*	0.00	_*
B/CPT 5	44A	SAND, medium - coarse, with a few gravels, sorted	0.18	0.44	3.0

Table 3: Particle Size Distribution

* Contains more than 10% silt/clay

5.4 Thermal Conductivity

The thermal conductivity was determined on sample 40A from borehole B/CPT 1, sample 43A from borehole B/CPT 4 and sample 26A from borehole B/CPT 5. The results of these teste are given in Table 4 and in Enclosure Nos. 1.10 - 1.12.

Table 4:	Thermal Con	hermal Conductivity								
Borehole No.	Sample No.	Depth (m b. g. l.)	Geological Description	Thermal Conductivity (W / (m*K))						
B/CPT 1	40A	20.0 - 20.6	CLAY, very high plasticity, micaceous	1.549						
B/CPT 4	43A	20.0	SAND, fine, micaceous, with small fragments of lignite, sorted	2.207						
B/CPT 5	26A	13.0 - 13.5	CLAY, very high plasticity, with many thin layers of silt	1.669						

The results of the conducted tests show that the thermal conductivity of the tested samples is in the range of 1.549 - 2.207 W/(m*K).

6 Water levels

The water level in the standpipes in the boreholes was measured the 29 March 2021. The results are presented in Table 5 below:

Borehole No.	Elev. (m)	Filter level (m b. g. l.)	Date	Water I	Level		
110.	()	(11 5. 9. 1.)		Depth (m b. g. l.)	Elev. (m)		
B/CPT 4	+2.7	4.4 - 5.0	29-03-2021	1.7	+1.0		
B/CPT 5	+5.1	5.3 - 6.0	29-03-2021	2.5	+2.6		

Table 5:Water level measurements 29 March 2021

The ground water level is expected to vary during the cause of the year due to seasonal variations.

A more detailed description of the encountered subsurface conditions can be seen on the borehole logs.

7 Geotechnical interpretations

The terrain at the boreholes and CPTs varies from +5.1 to +1.2 m.

Based on the boreholes and CPTs it is expected that the HDD will primarily be drilled in late glacial/glacial clay or in Miocene sand and clay deposits.

Field vane tests in the late glacial/glacial clay deposits show an undrained shear strength of approx. $100 - 300 \text{ kN/m}^3$. Field vane tests in the late Miocene clay deposits show an undrained shear strength of approx. $600 - >700 \text{ kN/m}^3$.

Furthermore, most of the late glacial, glacial and Miocene clay deposits have a very high plasticity, which is based on their water content and the visual classification of the samples

There is a potential risk of swelling for high and very high plasticity clay. If the HDD is placed in the late glacial, glacial and Miocene clay deposits, appropriate measures must be taken to ensure that the drilling equipment does not stop as a result of increased surface pressure from the clay.

The performed Atterberg limit tests shows Plasticity Indexes ranging from 6.6 to 33.5 as shown on the borehole logs Enclosure Nos. 1.1, 1.4, 1.5. indicating a lower plasticity. However, based on the water content and the visual classification of the samples, it is estimated that the potential risk of swelling will be that of very high plasticity clay.

The Miocene sand deposits at the western part of the line are mainly fine grained and silty. At the eastern part of the line the sand deposits are mainly fine – coarse grained. The SPT tests conducted in the Miocene sand deposits indicates, that these deposits are generally very dense.

The primary challenges for the HDD will be:

- > Risk of swelling of clays with high and very high plasticity
- > Very dense Miocene sand and clay deposits
- > Loss of pressure in sand and gravel layers

8 Final remarks

The recovered soil samples will be stored for a period of one year from the date of this report. After this period the samples will be disposed of unless other agreed with Energinet.

COWI is available for further discussions regarding geotechnical and foundation issues in relation to the project.

LEGEND	FOR BOREHC	LE LO	GS					
Soil Si	gnature			Lo	cation Plan	Borehole log		
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		Lin	Limestone		LB 10 Pump Boring - Environmental Boring	3 Sample number		
Mull		at	⊕⊕⊕⊕⊕	Simple Boring Stratigraphic Boring Geotechnical in situ boring and laboratory tests	Undisturbed sample Layer boundary Disturbed sample 10 kg disturbed sample or SPT sample			
$\begin{bmatrix} \bot \\ \bot $	Sand	Cla	y till	Ge	Sounding, dynamic penetratio test ological abbreviations			
Gyttja	Silt		t till nd till	Envi Fw Ss Fi Gl Ma Wd Ts Ls Mw Ae	ronment : Freshwater deposit Solifluction soil Fill Glacier deposit Marine deposit Washdown deposit Top soil Landslide deposit Meltwater deposit Aeolian (wind) deposit	96.11.06 Water level and date of observation Backfill Stand pipe Bentonite filling		
	can be expected boulders (not ap		avel till in borings	Re Pg Lg Gc Ig Te	Recent Postglacial Late Glacial Glacial Interglacial Tertiary Cretaceous Danian	Sand filling		
Definition								
Signature	Parameter		Sym.	Unit	Definition			
$\begin{array}{ c c c c c c } & & & & & & & & & & & & & & & & & & &$				% % % kN/m³ % % ~ kN/m² kN/m² kN/m² N200 N200	Weight of water in percent of Water content at yield point Water content at plastic limit $w_L - w_P$ Ratio between total weight and Weight loss of combustion in p gl - ka Weight of CaCo ³ in percent of Reaction w. hydrochoric acid: of lime; + = limy; ++ = high Shear strength measured with in undisturbed soil Shear strength measured with in disturbed soil No. of ½-rotations pr. 200 mm percent	total volume ercent of solid matter solid matter = no lime; (+) = low content content of lime a vane test a vane test penetration		
Sounding resistance Light penetration test Sounding resistance SPT-probe, closed/open		R _{LSD} SPT	N200 N300	No. of strokes pr. 200 mm pe No. of strokes pr. 300 mm pe				
Date 06.09.2002	Designed VKJ		Contro CTL	olled		G-A4-01 Page 1 of 1		
COW	Л				END FOR HOLE LOGS	Data sheet $A-1$		

n:\1703\STD-Business Development\Geoteknik\DATABLAD, GEO\A-I\Datablad_Al_eng.dgn









IS2020 20.03.36 PSTCPT1 16-04-2021 11:23:19







GIS2020 20.03.36 PSTCPT1 16-04-2021 11:23:24





GIS2020 20.03.36 PSTCPT1 16-04-2021 11:23:26









Depth (m)	Т	Test Results						Geology	Sample	No.	Geolo	ogical D	escript	ion	Env	Age	Frost	CaCO ₃
													Con	tinued				
[F 7	CAND	_ " _			5.			
:7- 						\mathbb{P}		Γ.Υ		57	SAND					/ Mi		-
-							-25			58	SAND	_ " _				/ Mi		-
28 []]										59	SAND	- " -			Fv	/ Mi		-
							_											
	0	10	20		30	W (%)					21 25 -	and 31: Va	mind					
	∇	12 100	<u>16</u> 200		20 300	γ (kN/m Crv,Cfv)	s	Sample		and 31: Ic		l lake dep	osits			
	\rightarrow	10 5	20 10		<u>30</u> 15	N (Blow qc (MPa	s/30		-				ooci= -					
									P	rojec	tion: UT	uger with FM32E89) Y: 625		Plan:				
ľ	Project:											investi	gations		-1. 5.1			
	Drilled by: Prepared b		IC			3.11 Geo KSV App						olic No.: :e: 2021	.04.16	Boreh	ole: B/0		. 4/4	
ŀ	CO	W												Bor	ehol			
G	eoGIS2020 20.03.36	PSTR 16-04-202	21 11:24:25														- 3	,





GeoGIS2020 20.03.36 PSTCPT1 16-04-2021 11:23:27











IS2020 20.03.36 PSTCPT1 16-04-2021 11:23:30





IS2020 20.03.36 PSTCPT1 16-04-2021 11:23:31





GeoGIS2020 20.03.26B GSD1 16-04-2021 11:13:32



GeoGIS2020 20.03.26B GSD1 16-04-2021 11:13:36













COWI A/S
 Jens Chr. Skous Vej 9
 8000 Aarhus C

Danmark

		PROJECT NO.	A224691	
		DRAW.	LNJE	
		CHECKED	HRMO	
		APPROVED	HRMO	
		SCALE	1:200 / 1:10	00
		DATE	2021.04.16	
	ENCLOSURE NO.			VERSION
Tlf +45 56 40 00 00 Fax +45 56 40 99 99 www.cowi.dk	1.13			1.0
IMech\Sheet\A224691-Profile_enclosure_1.13.dgn			21-04-20)21 12:07:09



B/CPT5 🐺 СРТ6-B/CPT4



Legend

X

Borehole with CPT and SPT

Borehole with CPT

Ф СРТ

VER.	DATE	REMARK

Energinet Eltransmission A/S Thor offshore wind farms. Landfall site investigations

Geotechnical Investigations Location plan

REMARKS



Enclosure 1.14

Coordinates on this drawing refer to UTM32 ETRS89 Vetical heights refer to DVR90				
	DRAW.	CHECKED	APPROVED	

		PROJECT NO.	A224691	
		DRAW.	LNJE	
		CHECKED	HRMO	
		APPROVED	HRMO	
		SCALE	1:1000	
		DATE	2021.04.16	
	ENCLOSURE N	١0.		VERSION
0 00 00 0 99 99	1.14			1.0