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HESSELØ OFFSHORE WIND FARM

SITE INVESTIGATIONS FOR HDD

GILBJERG HOVED



HESSELØ OFFSHORE WIND FARM SITE INVESTIGATIONS FOR HDD GILBJERG HOVED

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1. EXECUTIVE SUMMARY

This report has been prepared to provide geotechnical, geological, and geophysical data in order to disclose the basic soil conditions for the horizontal directional drilling (HDD) location of Gilbjerg Hoved. A risk assessment has been part of the process whereas potential risks has been identified and evaluated for the HDD.

In order to disclose the basic soil conditions, a site investigation campaign was carried out involving a total of 5 geotechnical boreholes performed on the expected HDD location to the depths from 25 to 50 meters below ground level. Furthermore, approximately 1310 meter of Multi Electrode profiling was carried out.

The HDD crossing will be performed in glacial deposits with a known risk of encountering obstructions such a boulders, cobbles and firm sand deposits. Additionally, the HDD crossing will be performed in saturated and unsaturated sand- and gravel deposits where sufficient pressure can be challenging to maintain during drilling. Furthermore, the HDD crossing should be expected to drill below- and above water table which can require adaptation to maintain sufficient pressure and set-up.

In connection with the design and planning of the HDD, calculations to evaluate the factor of safety against blow up of drilling fluid should be performed for the HDD crossing during the final design process. These calculations should be used to determine the maximum fluid pressure limitations and modify the alignment of the HDD, if necessary, to maintain appropriate factors of safety against blow up.

Furthermore, additional factors must be implemented to reduce the potential for blow up of drilling fluids, including the following:

- Preparing spill prevention, control, and countermeasure for the HDD crossing
- Establishing minimum requirements for HDD contractors
- Requiring HDD contractors to develop and follow fluid monitoring programs and blow up prevention plans.

2. INTRODUCTION

2.1 General

Rambøll has been appointed to perform the geophysical and geotechnical site investigation for a planned HDD at the location of Gilbjerg Hoved in the northern part of Zealand.

This report summarizes the results of the performed geophysical and geotechnical investigation.

This report provides recommendations and a risk assessment for tackling problems in the subsurface when using the horizontal directional drilling (HDD) technique for the crossing. HDDs will typically be considered where obstacles at the ground surface, such as sand dunes, protected areas, roads, railway, and water channel, make traditional cut and cover pipeline installation problematic, or where natural amenities such as the wet area are crossed.

This report contains a description of the proposed project, information on the HDD installation technique, a summary of the location where this technique is being considered and discussions concerning surface, geological, and geotechnical conditions at the location. An assessment of the risks for the successfully completing an HDD installation at the locations is provided based on all available information.

All boring locations and depths and sites for geophysical investigations have been selected and defined by Energinet.

2.2 Project description

2.2.1 Background

In June 2020 the Danish Parliament decided to commence the development of the offshore wind farm (OWF) project, Hesselø aiming for a capacity of ca. 1000 MW. It is planned to build and connect the OWF to the Danish onshore electrical grid.

The OWF site is located in the inner Danish Sea, Kattegat, and has been subject to screening studies. The area of investigation subject to the OWF spans an area of ca. 247km².

Following the political decision, the Danish Energy Agency has instructed Energinet to initiate site investigations, environmental and metocean studies and analysis for grid connection for the area of investigation.

On the basis of the instruction from the Danish Energy Agency, Energinet requests Rambøll to carry out a geotechnical and geophysical site investigation at the landing site of the export cable from the OWF prior to the planning and execution of an HDD (Horizontal Directional Drilling).

2.2.2 The project

The HDD process involves boring under a feature and pulling the pipeline into place through the borehole that has been reamed to accommodate the diameter of the pipeline.

This process has three main phases: pilot-hole drilling, subsequent reaming passes, and pipe pullback. These phases will not be described in further details since the techniques are assumed to be familiar to the contractors.

Figure 2-1 presents an overview map of the location. Figure 2-2 presents an overview map of the



Figure 2-1: Overview map of the location. Orange polygon: OWF area. Pink polygon: cable routes ending at the landing site at Gilbjerg Hoved west of Gilleleje.



Figure 2-2: Black dotted line: Planned HDD position, Gilbjerg Hoved.

2.3 Regional geological background

2.3.1 General geology

The landscape in the northern part of Zealand was created by a series of glaciations.

The surface soils in the northern Zealand are dominated by Clay Till, meltwater sand and gravel.

The Quaternary landscape of the northern Zealand is the result of a series of events in Late Weichsel glaciation. The period extends about 25.000 to 11.000 years back. During this period, three major glaciers formed the landscape present today. First The Kattegat Isstrøm (Kattegat Iceflow) from South Norway, followed by a mid-Sweden glacial advancement called NØ-isen (NE-ice) ending with a progression from East, The Østjyske Isstrøm. Several locations show how the ice has folded, stacked and pressed up the Kattegat Isstrøm and NØ-isen, forming elongated slightly twisted N-S directed moraines. Between these elongated moraines are valleys with meltwater deposits. These meltwater rivers flowed from south to north, draining large dead-ice areas around Esrum Sø. The imprints of these meltwater rivers are still seen today and are now known as Esrum Ådal and Pandehave Ådal.

A cross-section of Esrum Ådal is shown in Figure 2-3 where the Kattegat Isstrøm was responsible for the lowermost Till deposits and the NØ-isen, the upper most Till-deposits, the section is separated by meltwater and lake- deposits.

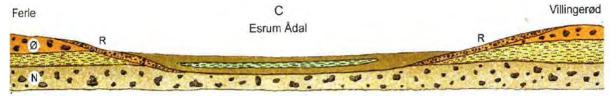


Figure 2-3: From Houmark-Nilsen, 2021 [4]. Terrain model of an area between Gilleleje and Esrum. \emptyset = Upper Till deposited by the NØ-isen/NE-ice. N = the Lower most Till deposited by the Kattegat Isstrøm/ Kattegat Iceflow. R = Pebble Gravel. N and \emptyset are separated by meltwater deposits.

2.3.2 Local geology

The survey area is located in an area dominated by meltwater deposits. The nearest archive boreholes in the area are >200 meters away from the current area of study. Due to the large distance, it won't be relevant to take this material into consideration.

The geological map, Figure 2-4, is a representation of the present geology in 1,0 m depth. As seen on Figure 2-4, the boreholes are all positioned in the meltwater deposit-zone, except for BH05 which is located near the coastline and therefore in a marine environment. As described above, the geological map is a representation only of the uppermost 1,0 m soil, and it can be expected, that the meltwater deposits that characterize the four other boreholes will define BH05 as well.

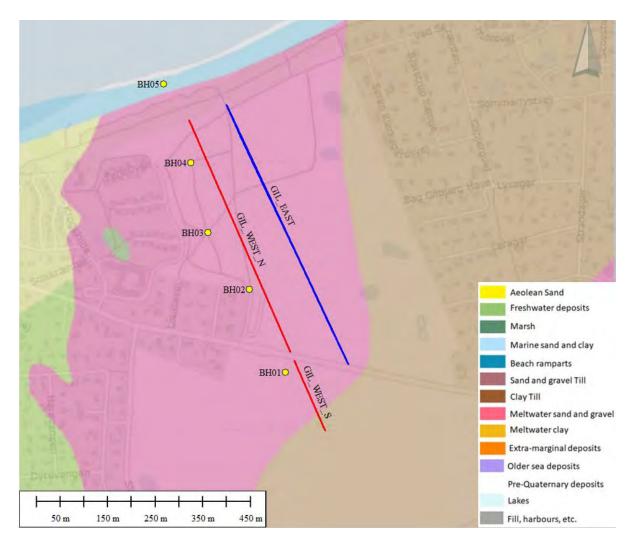


Figure 2-4: Geological map of the survey area including legend, borehole positions and (blue and red) MEP-interpretation lines

3. PURPOSE OF THE INVESTIGATION

The purpose of the site investigations is to investigate and document the geotechnical and geological conditions along with the possible lines for the HDD, so that:

- Informing the client and HDD suppliers about the ground conditions
- Decreasing the soil-related risks for performing HDDs
- Selection of HDD equipment feasible for performing the planned HDDs
- Design of the HDD underground trajectory

The work has been carried out in accordance with Eurocode EC7, Part 1, Part 2, with its associated Danish Annex, DS/CEN ISO/TS 17892 and the Danish Geotechnical Association's Bulletins.

4. EXTEND OF FIELD INVESTIGATIONS

4.1 General

In accordance with the program of the site investigation campaign, a total of 5 boreholes have been performed on site to depths from 25 to 50 meters below ground level (bgl). For this, a total of 160 borehole meters was completed. Furthermore approximately 1310 m Multi Electrode Profiling (MEP) was carried out on site.

Table 4-1 provides details about the five geotechnical boreholes.

Table 4-1: Boreholes drilled at the HDD-site

Borehole	Depth	Date	Projection	Х	Υ	LSYS	Z
[-]	[m]	[YYYYMMDD]	[-]	[-]	[-]	[-]	[m.
							DVR90]
HESS.HDD.BH01	25,00	2022-05-17	UTM32E89	703431	6224376	DVR90	+17,80
			ETRS89				
HESS.HDD.BH02	25,00	2022-05-03	UTM32E89	703355	6224550	DVR90	+20,39
			ETRS89				
HESS.HDD.BH03	35,00	2022-05-31	UTM32E89	703268	6224669	DVR90	+20,64
			ETRS89				
HESS.HDD.BH04	50,00	2022-05-23	UTM32E89	703233	6224815	DVR90	+30,64
			ETRS89				
HESS.HDD.BH05	25,00	2022-04-25	UTM32E89	703176	6224980	DVR90	+3,13
			ETRS89				

Per Aarsleff has carried out setting out of borehole positions in the field with GPS and with reference to the coordinate system ETRS89 UTM32N and DVR90.

Boreholes were performed by Per Aarsleff with the shell and auger method as **6" to** 8" drillings with casing. The geotechnical field exploration program was performed in the April and May 2022.

The geophysical measurement was carried out by Rambøll in April 2022.

An overview of legend and definitions are provided in Appendix 1.1.

Borehole location plans- and MEP location plans are provided in Appendix 2.1.

Geotechnical- and geophysical longitudinal profiles for respectively the western and eastern site investigation lines, are provided in Appendix 3.1-3.4.

4.2 Extend of geotechnical investigations

During drilling strata boundaries were recorded and soil samples were taken as bag samples (disturbed samples) and A-tubes for geological assessment, classification tests and thermal resistivity tests. Disturbed soil samples were collected at depths of 0.2 and 0.5 m and then generally every 0.5 m, and at least one sample per strata. If assessed possible A-tubes were extracted at an interval of 5 meter. In case of a soil stratum consisting of sand or gravel big bags have been collected instead of the A-tubes.

Field vane tests have been performed in cohesive soils (clay and silt) in accordance with Reference sheet no. 1 from Danish Geotechnical Society in order to evaluate the undrained shear strength (c_u , c_{ur}). Standard Penetration Tests have been performed in cohesionless layers in accordance with Reference sheet no. 3 from Danish Geotechnical Society in order to evaluate the relative density and the angle of internal friction (ϕ') in the sand.

A total of 4 standpipe piezometers (Ø25mm) have been installed in borehole BH01-BH04 for registration of the groundwater table. In borehole BH05, no standpipe piezometers have been installed as the borehole is located near the coastline and therefore sealed upon termination of the drilling.

A total of 5 attempts of extracting A-tubes were done, however, failed due to the tube being damaged in all the attempts.

The recovered samples have been geologically described in the laboratory. On all samples, the water content has been determined.

Boreholes were sealed with bentonite pellets after completion and just after the final round of ground water soundings were collected.

4.2.1 Laboratory tests

The laboratory works have included geological description of the soil samples in accordance with Danish Geotechnical Society Bulletin, denoted DGS, ref. [1].

Based on the site investigation results Ramboll prepared laboratory testing proposals for each borehole, which were approved by Energinet before commencement.

The program included classification tests as well as advanced laboratory tests:

- Determination of the moisture content in accordance with Danish Geotechnical Society Bulletin 15, section 3.1, approximately per meter.
- Particle size distribution including hydrometer testing in accordance with CEN ISO/TS 17892-4 performed on selected samples.

A limited amount of laboratory tests was performed due to the relatively homogenous soil conditions that were found in the geotechnical boreholes.

The borehole logs are provided in Appendix 4.1-4.5.

Laboratory test results from the particle size distribution are provided in Appendix 5.1-5.5.

4.2.2 Comments to geotechnical drilling works

Comments provided by the driller during the performed drilling works, concerning general drilling and complications such as milling and presence of stones or boulders, are presented in Table 4-2 below. For further detailed, refer to the field logs in Appendix 6.1-6.5.

Table 4-2: Overview of the comments to the geotechnical drilling works

Borehole	Depth	Comment
[-]	[m]	[-]
HESS.HDD.BH01	25,00	 Presence of very sandy Clay Till layers at the top of the strata. Presence of silty and gravelly sand layers embedded below the Clay Till layers, to the bottom of the strata. Vane test (V5) in Sand Till. measured maximum strength at 3,40 meters bgl. Vane test (V4) in Clay Till measured maximum strength at 4,40 meters bgl
HESS.HDD.BH02	25,00	 Presence of sand layers with gravel and stones throughout the strata. Milling from 13,00-13,25 meters bgl. Approximate milling duration, 3,5 hours. Milling from 14,00-14,25 meters bgl. Approximate milling duration, 2,0 hours. Milling from 16,10-16,36 meters bgl. Approximate milling duration, 1,0 hour. Milling from 17,30-17,50 depth bgl. Approximate milling duration, 1,0 hour.
HESS.HDD.BH03	35,00	Presence of stony, gravelly, and silty sand layers throughout the strata has slowed the borehole drilling.
HESS.HDD.BH04	50,00	 Presence of stony, gravelly, and silty sand layers throughout the strata has slowed the borehole drilling.
HESS.HDD.BH05	25,00	 Presence of stony, gravelly, and silty sand layers throughout the strata has slowed the borehole drilling.

Furthermore, all attempts of extracting A-tubes have resulted in the tube being damaged.

4.2.3 Ground water conditions

Standpipe piezometers have been installed in four out of five boreholes. The measured water levels are listed in the Table 4-3 below. The measured levels and standpipe piezometers can be seen in the borehole logs in Appendix 4.1-4.5.

Table 4-3: Measured water levels

Borehole	Pipe	Terrain	Date	Water depth	Water level
[-]	[mm]	[m. DVR90]	[YYYYMMDD]	[bgl]	[m. DVR90]
HESS.HDD.BH01	Ø25	+17,80	2022-05-17	15,60	+2,20
			Lost due to plowing	-	-
HESS.HDD.BH02	Ø25	+20,39	2022-05-03	18,62	+1,77
			2022-05-27	18,64	+1,75
HESS.HDD.BH03	Ø25	+20,64	-	-	-
			2022-06-10	18,97	+1,67
HESS.HDD.BH04	Ø25	+30,36	-	-	-
			2022-05-27	29,84	+0,52
HESS.HDD.BH05	-	+3,13	Sealed after termination of borehole	-	-

The water level was measured for BH01 on May 17th, 2022, and was observed at a level of +2,20m DVR90. No 2nd observation was made due to the loss of borehole and standpipe piezometer from plowing.

The water level was measured for BH02 on May 3^{rd} , 2022, and was observed at a level of +1,77m DVR90. The 2^{nd} observation on May 27^{th} , 2022, showed a water level at +1,75m DVR90.

The water level was measured for BH03 on June 10^{th} , 2022, ten days after completion of the borehole, and was observed at a level of $\pm 1,67m$ DVR90.

The water level was measured for BH04 on May 27th, 2022, fourteen days after completion of the borehole, and was observed at level +0,52m DVR90.

The water level was not measured for BH05 due to no standpipe piezometer installation as the borehole is located near the coastline and therefore sealed upon the termination of the drilling.

The observation of the measured water levels shows a steady decrease towards the coastline.

4.3 Extend of geophysical investigations

In the geotechnical realm, geophysical methods are valuable in mapping the subsurface, such as including the ability to collect data over large areas in a relatively short period of time. This survey includes the MEP method (Multi electrode profiling) - a geoelectrical geophysical method, whereby it is possible to determine the resistivity of the subsurface.

The method is also called ERT (electrical resistivity tomography) and CVES (continuous vertical electrical sounding) but will be called MEP in this report.

4.3.1 MEP

The resistivity measurements were acquired as MEP with a roll-along technique. The survey lines were acquired with cables with an electrode spacing of 2 m south of Tinkerup Strandvej and with

an electrode spacing of 5 m north of Tinkerup Strandvej. Steel electrodes fastened to the ground were attached to the electrodes on the cables. For each measurement 4 electrodes are actively used, 2 electrodes emit the direct current, and 2 receiving electrodes measure the potential difference between the electrodes in the subsurface. The electrode configuration was set to GradientXL and was measured using a GuidelineGeo Terrameter LS2 instrument. Resistivities are measured in ohm.m.

This method results in a 2D resistivity profile along the measured line. Resistivity is a measure of a materials ability to conduct an electrical current. It is therefore possible to differentiate between materials with low resistivity such as clays, medium resistivity such as saturated sediments, medium-high resistivity such as unsaturated sediments and materials with high resistivity such as very dry coarse-grained sediments or bedrock.

The results from the measurements are presented in section 6 and the technical drawings are presented in Appendix 3.1-3.4

4.4 Results

The siteplan and the longitudinal profiles for the HDD site are provided in Appendix 3.1-3.4

Geotechnical borehole logs are provided in Appendix 4.1-4.5, including site investigation results and laboratory results from moisture content and bulk density/unit weight.

Laboratory test results from particle size distributions are provided in Appendix 5.1-5.5.

Field logs are provided in Appendix 6.1-6.5.

The risk analyses are provided in appendix 7.1.

5. HDD CROSSING CONDITIONS

Surface and subsurface conditions at the proposed HDD crossing location are provided in this section. Discussions of subsurface conditions include information from published geological mapping, groundwater observations, and conditions observed in geotechnical boreholes. Based on this information, an interpretation of the geological and geotechnical conditions at the HDD location is provided.

5.1 Ground properties

For the HDD construction both lower and upper values of strength parameters are required. The low characteristic values of strength parameters will mainly be needed for design of possible temporary constructions like sheet pile walls, containment dams or possible foundations etc. The high values of the strength parameters are mainly needed for the HDD contractor to assess the method and suitability of the drilling equipment, to assess the time for drilling and estimate the abrasion of the bit and drill rods for the job and to estimate pullback forces.

Proper selection of reamer and the number of passes needed depends on soil conditions, the hole size and pump capacity.

For installation in quaternary deposit (till and sand) as described in the boreholes a substantial wear (the borehole drilling works has been challenged by the ground conditions) on machinery should be expected. Enlarging the hole size to much in one pass will wear on the machinery. A reamer that is too large will result in excessive torque and pullback loads.

For the site, the intervals (minimum value and maximum value) for the measured (or derived) strength- and deformation parameters for each soil type have been provided in a table with ground properties.

In the tables below the following symbols apply:

γ : Soil unit weight – utilized above water level

 γ : Submerged soil unit weight – utilized below water level

c_u : Undrained shear strength (short term situation)

 φ : Effective angle of internal friction (long term situation)

c' : Effective cohesion (long term situation)

5.1.1 Geotechnical investigations

The geotechnical investigations include several tests performed in the encountered soil types in borehole BH01-BH05. Table 5-1 below, presents an overview of the field- and laboratory tests performed.

Table 5-1: Overview of tests performed in the encountered soil types in borehole BH01-05

Soil	Type	No. Of Shear Vane tests	No. Of MC tests	No. Of SPT tests	No. Of particle size
					distribution
[-]	[Env., Age]	[No]	[No]	[No]	[No]
FILL	Fi, Re	0	7	0	0
SAND	Mw, Lg	0	12	2	1
GRAVEL	Mw, Gc	0	12	3	1
SAND	Mw, Gc	5	191	75	25
GRAVEL TILL	GI, Gc	0	11	4	1
SAND TILL	GI, Gc	0	57	14	3
CLAY TILL GI, Gc		6	9	1	0
То	tal	11	299	99	31

The sections below, present a general description of the encountered soil types and present the interpreted results for each soil type encountered.

5.1.1.1 FILL, Fi, Re

In borehole BH01, BH02 and BH03, fill has been observed in the top of the strata as sandfill, fine – medium, mully, slightly gravelly, slightly silty dark yellowish brown, and not calcareous. In some samples, plant remains have been observed. Laboratory tests have been performed in the fill layers and below are the results summarized and depicted as a range and average value in Table 5-2. No field tests have been performed in the fill layers.

Table 5-2: Laboratory test results of FILL, Fi, Re

Test	Minimum measured value	Maximum measured value	Average value	Number of tests
Moisture content [%]	5,91	10,11	8,18	7
Particle size distribution [No]	-	-	-	0

5.1.1.2 SAND, Mw, Lg

In borehole, BH02 and BH05, lateglacial meltwater sand has been observed in the upper layers near the surface as fine – coarse, slightly silty, a few grains of gravel, dark yellowish brown and slightly calcareous. Laboratory and field tests have been performed in the lateglacial meltwater sand layers and below are the results summarized and depicted as a range and average in Table 5-3. Figure 5-1 presents an overview of the particle size distribution based on 1 test from lateglacial meltwater sand in borehole BH05.

Table 5-3: Laboratory and field test results of sand SAND, Mw, Lg

Test	Minimum measured value	Maximum measured value	Average value	Number of tests
Moisture content [%]	6,10	15,88	12,84	12
SPT [No.]	11	12	11,5	2
Particle size distribution [No]	-	-	-	1

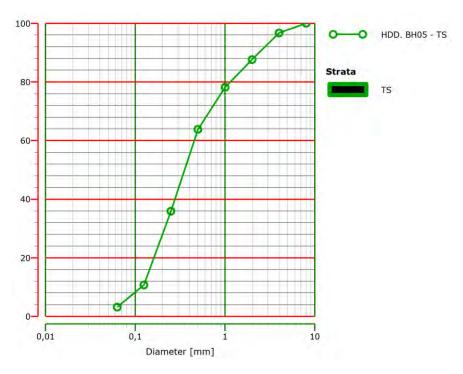


Figure 5-1: Particle size distribution of lateglacial meltwater sand in borehole BH05

5.1.1.3 GRAVEL, Mw, Gc

In borehole BH02 and BH04, glacial meltwater gravel, has been observed in the deeper layers as coarse, sandy, poorly graded, grey, and calcareous. Laboratory and field tests have been performed in the glacial meltwater gravel layers and below are the results summarized and depicted as a range and average in Table 5-4. Figure 5-2 presents an overview of the particle size distribution based on 3 tests from glacial meltwater gravel in borehole BH02 and BH04.

Test	Minimum measured value	Maximum measured value	Average value	Number of tests
Moisture content [%]	2,51	12,60	7,20	12
SPT [No.]	12	52	35	3
Particle size distribution [No]	-	-	-	3

Table 5-4: Laboratory and field test results of GRAVEL, Mw, Gc

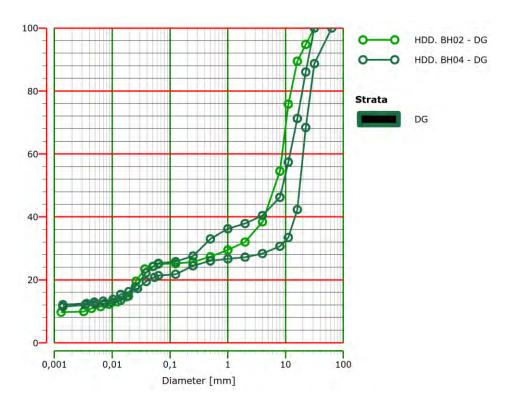


Figure 5-2: Particle size distribution of glacial meltwater gravel in borehole BH02 and BH04

5.1.1.4 SAND, Mw, Gc

In borehole BH01 and BH02, glacial meltwater sand has been observed in the deeper layers as fine – coarse, slightly gravelly, slightly silty, grey, and calcareous. In borehole BH03, BH04 and BH05, glacial meltwater sand has been observed in the upper layers close to the surface, as well as the deeper layers, as fine – medium, sorted, poorly graded, well graded, slightly silty, slightly clayey, dark yellowish brown, brownish grey, calcareous, and slightly calcareous. Laboratory and field tests have been performed in the glacial meltwater sand layers and below are the results summarized and depicted as a range and average in Table 5-5. To be noted, 5 shear vane tests has been performed in layers of glacial meltwater sand. Due to shear vane testing in frictional material, the results are not included to determine the strength or deformation of glacial meltwater sand.

Table 5-5: Laboratory and field test results of SAND, Mw, Gc

Test	Minimum measured value	Maximum measured value	Average value	Number of tests
Moisture content [%]	0	21,63	9,33	191
N _{SPT} [No.]	6	100	39	75
Shear Vane [kPa]	1	-	ı	5
Particle size distribution [No]	-	-	-	23

Due to the relatively large number of performed SPT tests in glacial meltwater sand, a statistical approach is used to analyze the plane friction angle, whereby the N_{SPT} values from field tests in borehole BH01-BH05 are derived into plane friction angles in accordance with DS415 and DS/EN [3]. The friction angles represent a conservative estimate of the mean (which is hereafter referred to as the lower average), which represents the average of the values that fall within a 95% confidence interval.

The data population for glacial meltwater sand, can best be approximated by a log-normal distribution. To calculate a conservative estimate of the lower average value, a log-normal distribution is used.

As presented in Figure 5-3, the lower average value of the plane friction angle of glacial meltwater sand is derived to be $\phi_{pl} = 39^{\circ}$.

To be noted, the glacial meltwater sand layers have generally been observed with no significant quantities of silt. Therefore, in accordance with DS415 and DS/EN, no deduction of the derived plane friction angle of glacial meltwater sand has been done.

Figure 5-4 presents an overview of the particle size distribution based on 23 tests from glacial meltwater sand layers in borehole BH01-BH05. As presented, BH01-BH05 has silt in its glacial meltwater sand deposits.

To examine the possible relation between depth and silt content in the layers of glacial meltwater sand, the data has been processed whereas particles from the 23 tests passing through 0,063 mm sieve are defined as silt (or clay).

Figure 5-5 presents an overview of the relation between depth and silt content of glacial meltwater sand in borehole BH01-BH05. As presented, the majority of the tests show a small content of silt, whereas no relative relation between depth and silt content were found. However, it should be noted that large amounts of silt were found in borehole BH04, as presented in Figure 5-5. Thus, in accordance with DS415 and DS/EN, it should be considered to deduct the derived plane friction angle in glacial meltwater sand according to the design purpose.

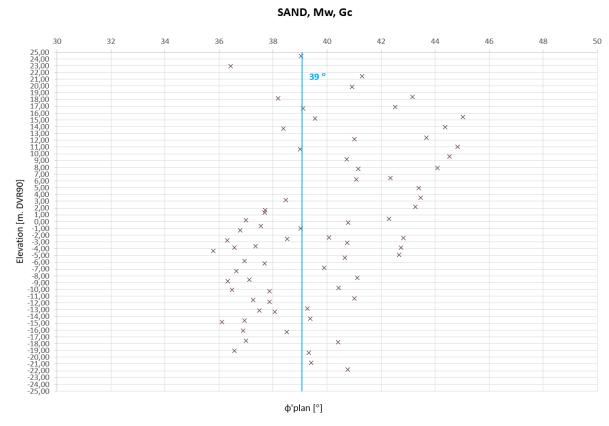


Figure 5-3: The lower average value of the plane friction angle of glacial meltwater sand

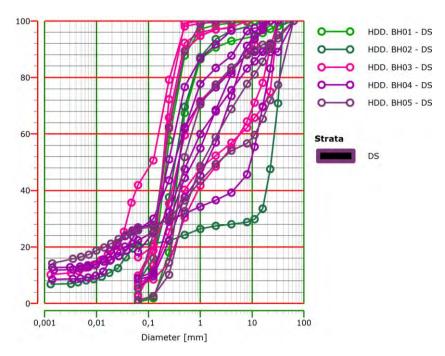


Figure 5-4: Particle size distribution of glacial meltwater sand in borehole BH01-BH05

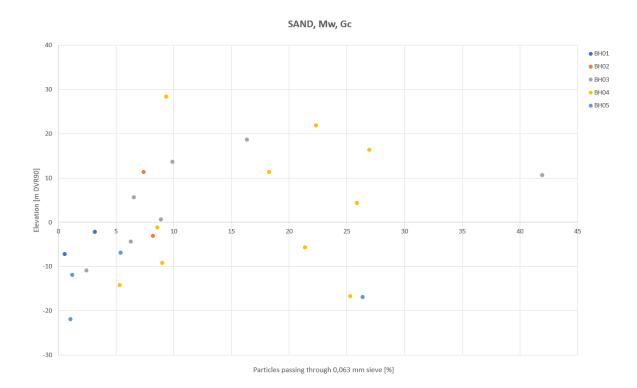


Figure 5-5: Particle size distribution of particles passing through 0,063 mm sieve of glacial meltwater sand in borehole BH01-BH05

5.1.1.5 GRAVEL TILL, GI, Gc

In borehole, BH02, glacial gravel till has been observed in the upper layer as well as the deeper layer as sandy, silty, grayish brown and calcareous. Laboratory and field tests have been performed in the glacial gravel till, layers and below are the results summarized and depicted as a range and average in Table 5-6. Figure 5-6 presents an overview of the particle size distribution based on 1 test from glacial gravel till in borehole BH02.

Table 5-6: Laboratory and field test results of GRAVEL TILL, GI, Gc

Test	Minimum measured value	Maximum measured value	Average value	Number of tests
Moisture content [%]	5,74	17,64	10,16	11
SPT [No.]	27	100	62	4
Particle size distribution [No]	-	-	-	1

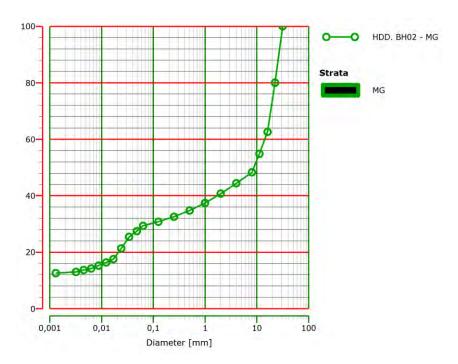


Figure 5-6: Particle size distribution of glacial gravel till in borehole BH02

5.1.1.6 SAND TILL, GI, Gc

In borehole BH01 and BH02, glacial sand till has been observed in the upper layers as well as the deeper layers as silty, gravelly, slightly clayey light yellowish brown, olive grey, not calcareous in the upper layers, however calcareous in the deeper layers. Laboratory and field tests have been performed in the glacial sand till layers and below are the results summarized and depicted as a range and average in Table 5-7.

Table 5-	·/: Laborato	ory and field	test results	of SAND III	∟L, GI, Gc
----------	--------------	---------------	--------------	-------------	------------

Test	Minimum	Maximum	Average	Number of
	measured	measured value	value	tests
	value			
Moisture content [%]	2,11	17,64	7,22	57
SPT [No.]	37	100	62	14
Particle size distribution [No]	-	-	-	3

Due to the relatively large number of performed SPT tests in glacial sand till, a statistical approach is used to analyze the plane friction angle, whereby the N_{SPT} values from field tests in borehole BH01-BH05 are derived into plane friction angles in accordance with DS415 and DS/EN [3]. The friction angles represent a conservative estimate of the mean (which is hereafter referred to as the lower average), which represents the average of the values that fall within a 95% confidence interval.

The data population for glacial sand till can best be approximated by a log-normal distribution. To calculate a conservative estimate of the lower average value, a log-normal distribution is used.

To be noted, glacial sand till, layers have been observed with smaller quantities of silt, and therefore it is assumed that the glacial sand till contains approximately 10% silt. Thus, in accordance with DS415 and DS/EN, 2 degrees is deducted from the derived plane friction angle of glacial sand till.

As presented in Figure 5-7, the lower average value of the plane friction angle of glacial sand till, is found to be $\phi_{pl}=40^{\circ}$.

Figure 5-8 presents an overview of the particle size distribution based on 3 tests from glacial sand till in borehole BH01 and BH02.

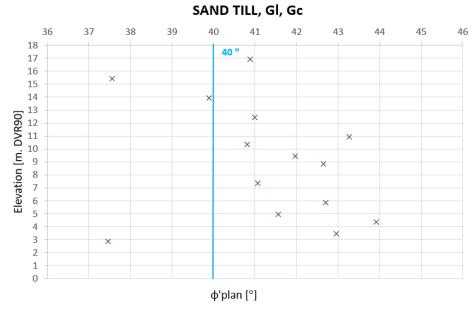


Figure 5-7: The lower average value of the plane friction angle of glacial sand till

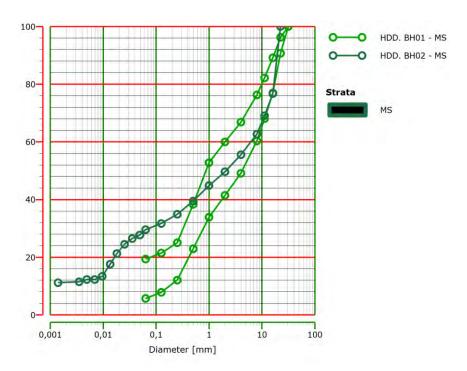


Figure 5-8: Particle size distribution of glacial sand till in borehole BH01 and BH02

5.1.1.7 CLAY TILL, GI, Gc

In borehole BH01, BH02 and BH03 glacial clay till has been observed in the upper layers as silty, very sandy, gravelly, with sand layers, grayish brown, not calcareous in the upper layers, however calcareous in the deeper layers. Laboratory and field tests have been performed in the glacial clay till, layers and below are the results summarized and depicted as a range and average in Table 5-8.

Table 5-8: Laboratory and t	field test results of	f CLAY TILL, GI, Gc
-----------------------------	-----------------------	---------------------

Test	Minimum	Maximum	Average	Number of
	measured	measured value	value	tests
	value			
Moisture content [%]	11,01	21,95	13,66	9
SPT [No.]	48	48	48	1
Shear Vane [kPa]	72	702	392	5
Particle size distribution [No]	-	-	-	0

Due to the relatively low number of performed shear vane tests, the strength- and deformation parameters in glacial clay till, must be determined on the basis of empirical knowledge and reference values.

To be noted, the five shear vane tests performed in glacial clay till, have been observed with a generous amount of sand in its deposit which can have an influence the shear vane test results.

The undrained shear strength is determined to match the average strength value, $c_u = 392 \text{ kPa}$.

The effective cohesion, c', is determined by the following expression: $c' = \frac{c_u}{10}$, whereas c' is limited to a maximum strength of 20 kPa. Hereby the effective cohesion, c' = 20 kPa.

The consolidation module is determined by the following expression that is based on the relation between the undrained shear strength, c_u , and the water content: $K = \frac{40}{w} \cdot c_u$. Hereby the consolidation module, K = 115 MPa.

5.1.1.8 Summary

Table 5-9 below summarizes the strength- and deformations parameters in the encountered soil types in borehole BH01-05. These parameters are determined on the basis of derived field-, and laboratory tests and on the basis of empirical knowledge and reference values.

Table 5-9: Summary of strength and deformation parameters of the encountered soil types in borehole BH01-05

Soil	Туре	γ/γ'	$\boldsymbol{\varphi_k}^1$	c'	Cu	K
[-]	[Env., Age]	[kN/m³]	[°]	[kPa]	[kPa]	[MPa]
FILL	Fi, Re	17/10	37	-	-	3-10
SAND	Mw, Lg	20/10	37	0	-	30
GRAVEL	Mw, Gc	21/10	37	0	-	40
SAND	Mw, Gc	20/10	39	0	-	40
GRAVEL TILL	GI, Gc	21/10	38	0	-	40
SAND TILL	GI, Gc	21/10	40	0	-	40
CLAY TILL	GI, Gc	21/10	30	20	392	115

In friction material the friction angle is plane, and I cohesive material the friction angle is triaxle

6. GEOPHYSICAL INVESTIGATIONS - MEP

The survey lines are called GIL-WEST and GIL-EAST are presented in Figure 6-1 below. Furthermore, details about the geophysical investigation and its survey lines are presented in Table 6-1.

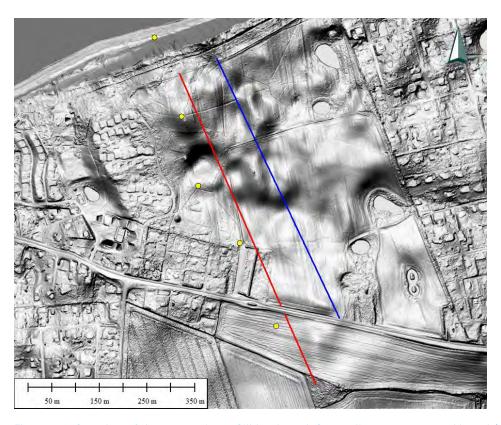


Figure 6-1. Overview of the survey site at Gilbjerghoved. Survey lines are presented in red (GIL-WEST) and blue (GIL-EAST). Boreholes are presented in yellow.

Table 6-1: Details of the geotechnical investigations and its survey lines

Survey Line	Colour	Surface Distance	Acquisition date	Х	Y
[-]	[-]	[m]	[YYYYMMDD]	[UTM32 ETRS89]	[UTM32 ETRS89]
GIL-WEST_N	Red	530	2022-04-12	North 703228.478	North 6224903.984
				South 703440.338	South 6224419.421
GIL-WEST_S	Red	160	2022-04-12	North 703448.694	North 6224401.101
				South 703513.215	South 6224254.842

GIL-EAST	Blue	600	2022-04-11	North 703306.329	North 6224936.127
				South 703562.323	South 6224394.279

The MEP data has been processed in the software RES2DINV and Aarhus Workbench. The resistivity measurements have been cleaned for unusually low and high resistivities and afterwards inverted with topography included. The topography has been extracted from the Danish Digital Terrain Model (0,4 m grid) from SDFE. During the data processing both single- and multi-layered 1D interpretations are made as well as 2D interpretations.

The results from the geoelectrical mapping have been correlated with the boreholes extracted from GeoGIS to investigate the soil condition between the boreholes. In the technical drawings the boreholes are orthogonally placed on the profiles labeled with an offset distance.

The MEP profiles are shown in colours representing resistivities in ohm.m according to the logarithmic colour scale presented in Figure 6-2 below.

The MEP Profiles are presented in Figure 6-3 and Figure 6-4 and in Appendix 2.1 and Appendix 2.2 and are oriented north to south.

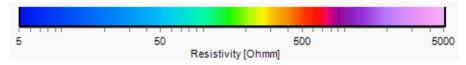


Figure 6-2. Logarithmic colour scale representing resistivities in ohm.m.

GIL-WEST

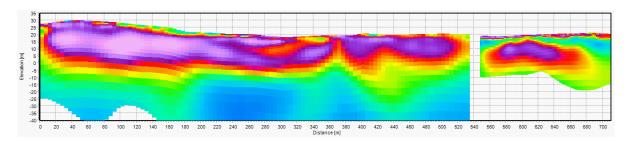


Figure 6-3. MEP profile GIL-WEST oriented north to south.

The profile is divided at the road crossing at 540 meters along the profile. The electrode spacing is 5 m north of the road and 2 m spacing south of the road. The difference is evident with a shallower penetration but with a higher resolution when 2 m spacing is used.

The sharp transition from high resistivities to low resistivities are related to the water table, thus it marks the boundary between unsatured and saturated sediments. The very low resistivities (dark blue) will normally be associated with clays, however this cannot be confirmed from the boreholes. It would not be surprising to find clays at the deepest part of the profile (-40 m), however it has not been interpreted.

According to the LER-database the profile is crossing a cable at approximately 370 meters along the profile. Undulating resistivity data correlate with position of 3rd party power cable.

From the boreholes the geology is mainly composed of sand and gravel throughout the profile but with mixed-in clay particles in the southern part. The resistivities are correlating with the labinterpreted geology, however a distinction between sand a gravel with or without mixed-in clay particles cannot be made from the resistivities.

North of Tinkerup Strandvej there are areas with clay till as seen in the low resistivities (greenish blue). This correlates with the boreholes. South of Tinkerup Strandvej the profile is situated in an agricultural field. The upper high resistivities are related to the agricultural soil. Below is a layer of clay till varying in thickness from approximately 1-10 m.

GIL-EAST

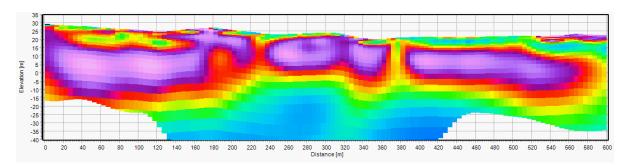


Figure 6-4. MEP profile GIL-EAST oriented north to south.

The profile is only acquired north of the road with an electrode spacing of 5 m.

The same sharp transition from high resistivities to low resistivities is present marking the boundary between unsaturated and saturated sediments. There are also very low resistivities in the deepest part of the profile normally associated with clays, however, as with the previous profile, it has not been interpreted as it could not be confirmed from the boreholes.

In the northern part of the profile there are areas of clay till as seen in the low resistivities (greenish blue). In the southern part of the profile a layer of clay till is interpreted with low resistivities and sharp boundaries to the unsaturated sediment below. In the southernmost part of the profile an unsaturated sediment is interpreted with high resistivities overlying the interpreted clay till.

The boreholes are further away and correlation becomes more uncertain with distance, however the resistivities correlate well with the lab-interpreted geology and there is a high correlation between the two profiles. It is however evident that the eastern profile has lower resistivities in the upper part which indicate a greater presence of clay till. This correlates well with the geological map (jordartskort 1:25000) from GEUS as presented in Figure 2-4. The brown colour is glacial clay till and the purple colour is glacial meltwater sand and gravel.

According to the LER-database the profile is crossing a cable at approximately 380 meters along the profile. Undulating resistivity data correlate with position of 3rd party power cable.

7. RISK ASSESMENT

The scope of this section is to establish a risk profile for the horizontal directional drillings. The purpose is to identify risks towards the contractor's successful completion of the project on time. Focus is on soil related risks.

Risks have been identified and evaluated in advance by geotechnical engineers as well as geologists and discussed and clarified in a workshop with Energinet. Although all efforts have been made to identify all relevant risk and evaluate them, no guarantees are given that all risk scenarios are covered and that risks are correctly assessed.

Risks are evaluated on the impact they will have on the project objective - time. The probability of occurrence is evaluated on a six-point scale ranging from Theoretical (<1%) to Often (>50%). Consequence on time is also evaluated on a six-point scale calibrated to a project with a relatively short duration, see Figure 7-1.

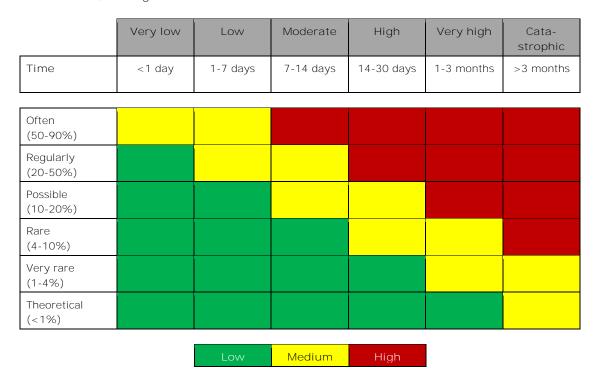


Figure 7-1. Risk matrices for evaluating impact on project time.

In Figure 7-2, the results are plotted in the risk matrix above.

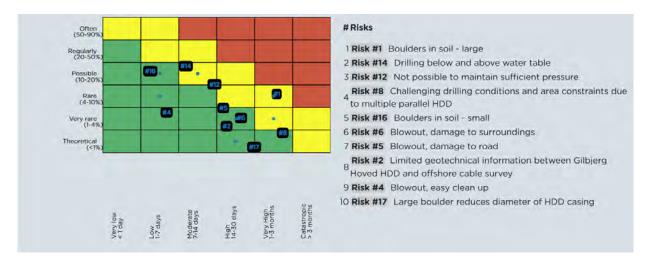


Figure 7-2. Risks plotted in risk matrix.

Below is an overview of the identified risks and the risk level. The risks are ranked with the highest risk first.

In Appendix 7.1 the risks, the effects and possible mitigation measures are described in more details.

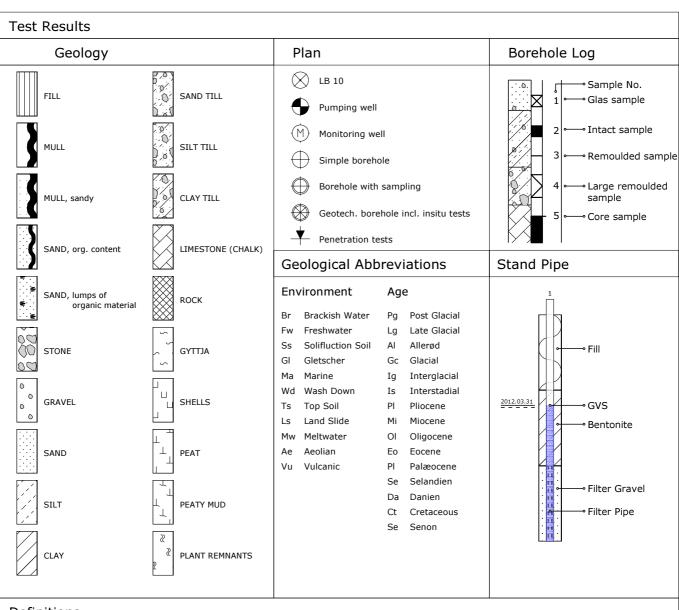
No.	Description	Risk Ievel
1	Boulders in soil - large	0
12	Not possible to maintain sufficient pressure	0
14	Drilling below and above water table	0
8	Challenging drilling conditions and area constraints due to multiple parallel HDD	0
16	Boulders in soil - small	0
2	Limited geotechnical information between Gilbjerg Hoved HDD and offshore cable survey	0
5	Blowout, damage to road	0
6	Blowout, damage to surroundings	0
4	Blowout, easy clean up	0

17	Large boulder reduces diameter of HDD casing
11	Risk of friction during drilling
15	Crossing of high voltage cable.
10	Interference with roots

8. REFERENCES

- [1] Danish Geotechnical Society Bulletin 1, revision 2, A guide to engineering geological description. 2021.
- [2] DS/EN 1997-1 DK NA: 2021, Eurocode 7: Geotechnical design, Part 1.
- [3] Fundering, Dansk Standard, DS415, 3. Edition. 1984.
- [4] Geologi og Landskaber i Nationalpark Kongernes Nordsjælland, M. Houmark-Nielsen. 2019.
- [5] DS/EN 1997-2 DK NA: 2013, Eurocode 7: Geotechnical design, Part 2.

APPENDIX 1.1 - LEGEND AND DEFINITIONS

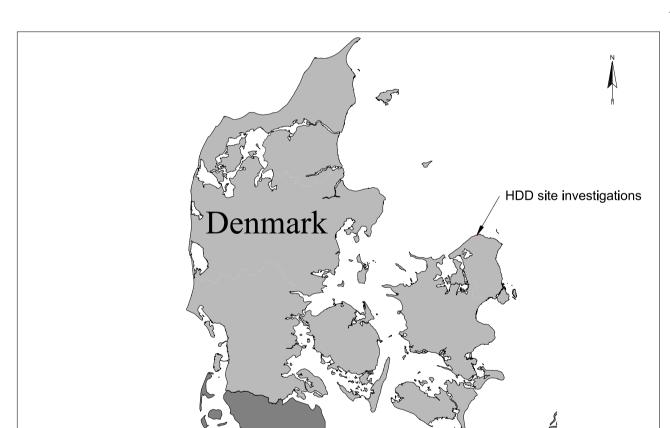


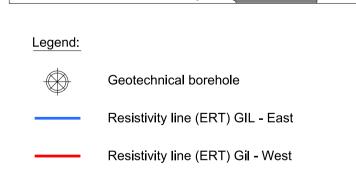
Definitions

Legend	Topic	Abbr.	Unit	Definition
Legena	Торіс	יוטטו.	Offic	Definition
0	Moisture Content	w	%	Water as % of dry weight
—	Liquid limit	WL	%	Water content at liquid limit
—	Plasticity limit	WP	%	Water content at plastic limit
Н	Plasticity index	IP	%	WL - WP
∇	Bulk Weight	g	kN/m³	Total weight of total volume
×	Loss of Ignition	gl	%	Loss of ignition as % of dry weight
×	Loss of Ignition, reduced	glr	%	gl - ka
⊕	Carbonate content	ka	%	Weight of CaCo3 as % of dry weight
-/(+)/+/++	Chalk Sample	kp	-	HCl Reaction: - not calcareous, (+) slightly calcareous, + calcareous, ++ very calcareous
++/+/(+) -//?/-?/+?	Frost			++ Freezing hazard under all conditions + Freezing problems, even during short periods of frost (+) Freezing problems, during long periods of frost - No Freezing problems Absolutly no freezing hazard ? Freezing hazard cannot be evaluated -?/+? Freezing hazard is difficult to evaluate
•	Vane shear strength, intact	cvf	kN/m²	Undrained shear strength - Vane test in intact soil
0	Vane shear strength, remoulded	cvr	kN/m²	Undrained shear strength - Vane test in remoulded soil
	Penetration Test:			
	- Weight Sounding Test	WST	N200	Number of half rotations per. 200 mm penetration
	- Light Sounding Test	LST	N200	Number of blows per 200 mm penetration
	- Light Dynamic Penetrometer	LDP	N200	Number of blows per 200 mm penetration
▼	- SPT, closed/open	SPT	N300	Number of blows per 300 mm penetration

APPENDIX 2.1 - LOCATION MAP







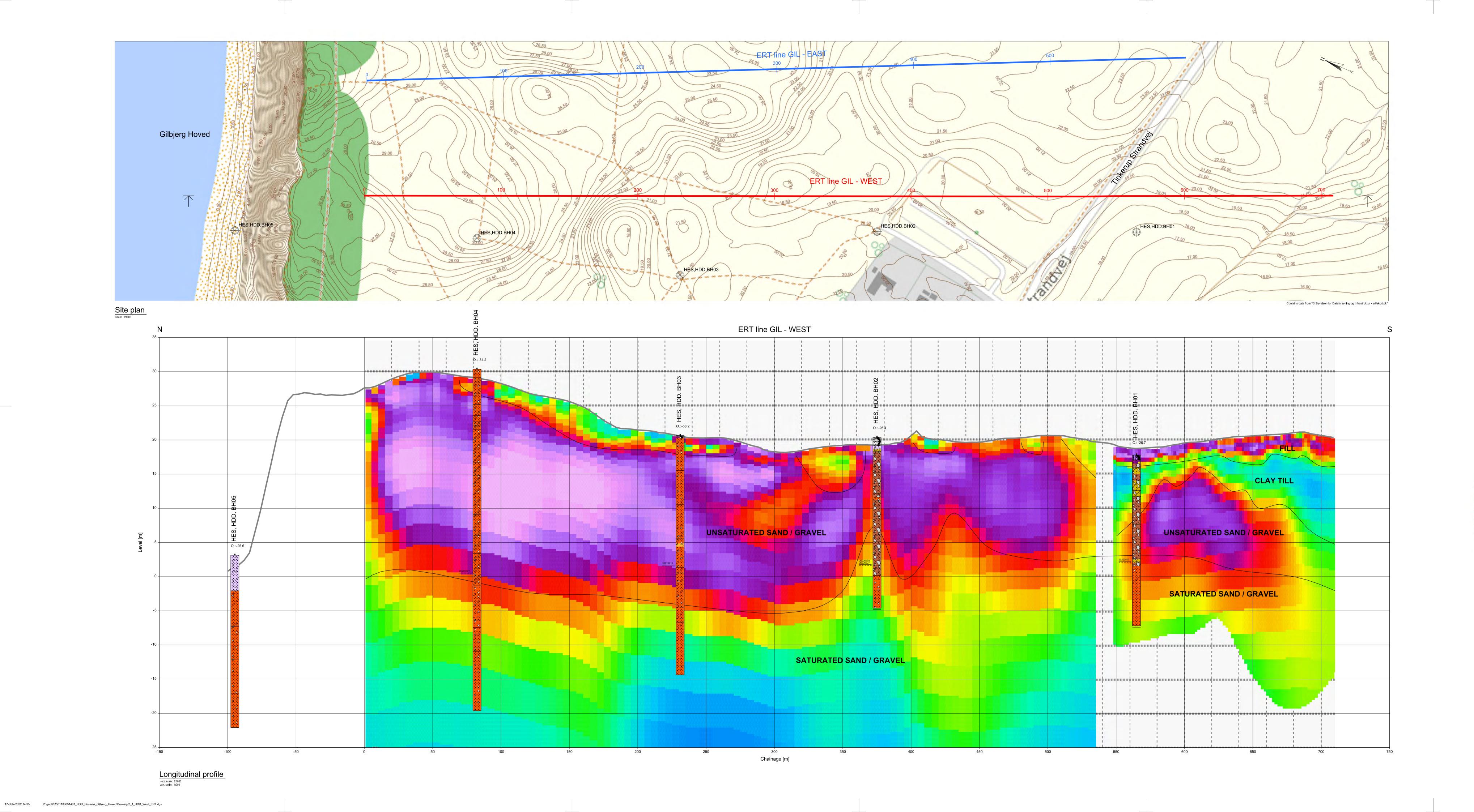
Contains data from "© Styrelsen for Dataforsyning og Infrastruktur - sdfekort.dk"

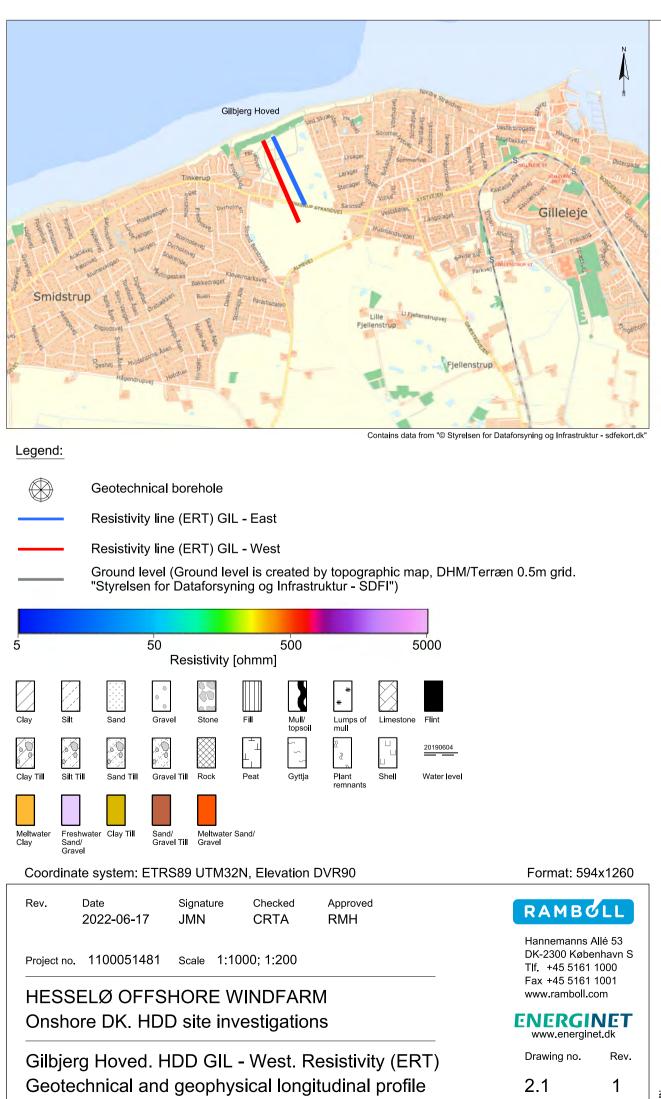
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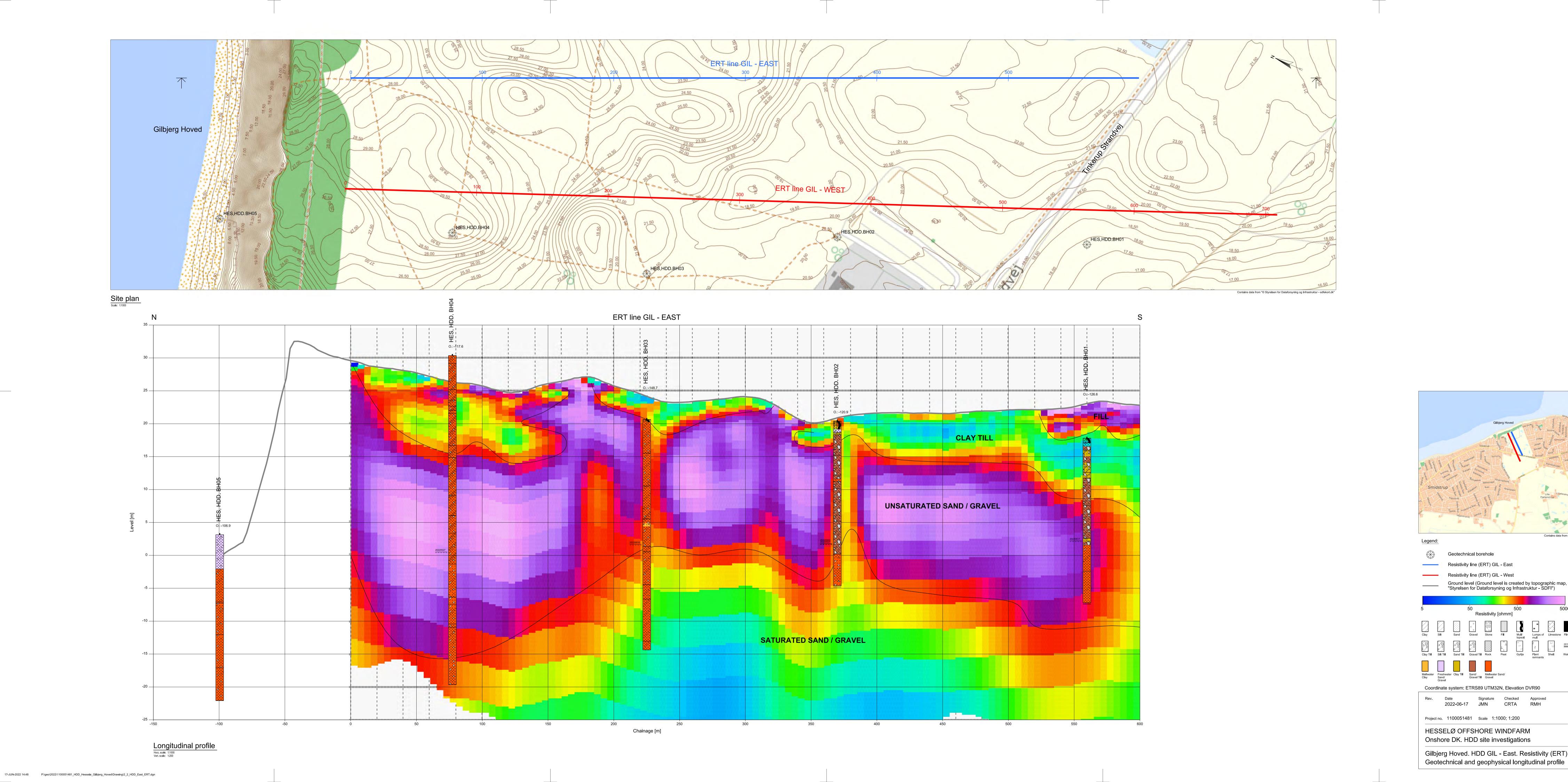
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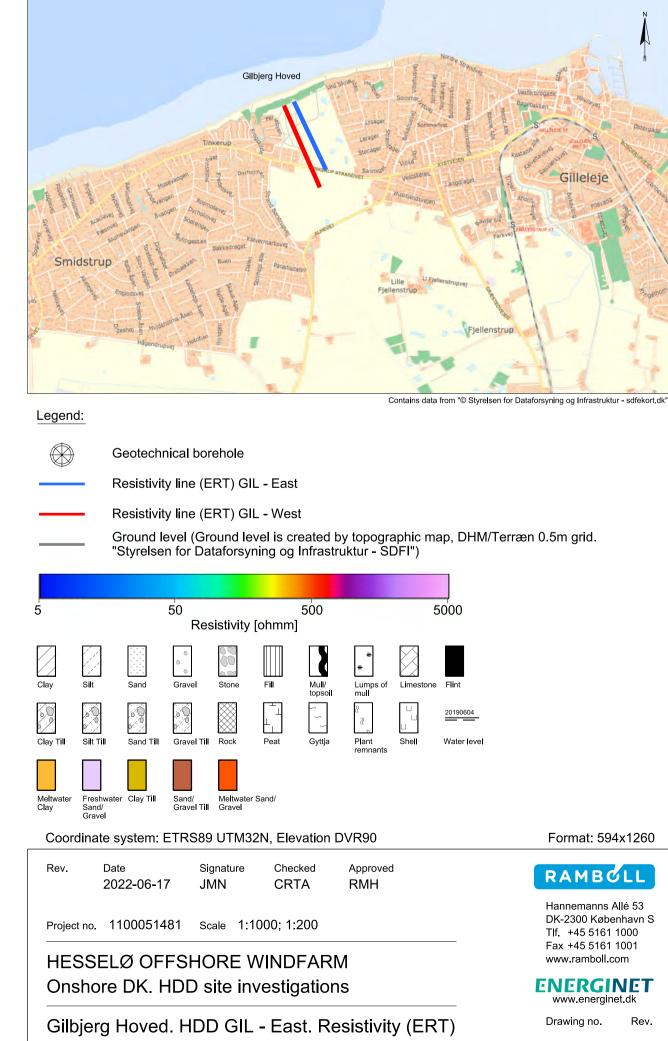
Rev.	Date 2022-06-09	Signature JMN	Checked CRTA	Approved RMH	RAMBOLL
Project r	по. 1100051481	Scale 1:20	000		Hannemanns Allé 53 DK-2300 Københavr Tlf. +45 5161 1000 Fax +45 5161 1001
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APPENDIX 3.1-3.4 - GEOTECHNICAL AND GEOPHYSICAL LONGITUDINAL PROFILES

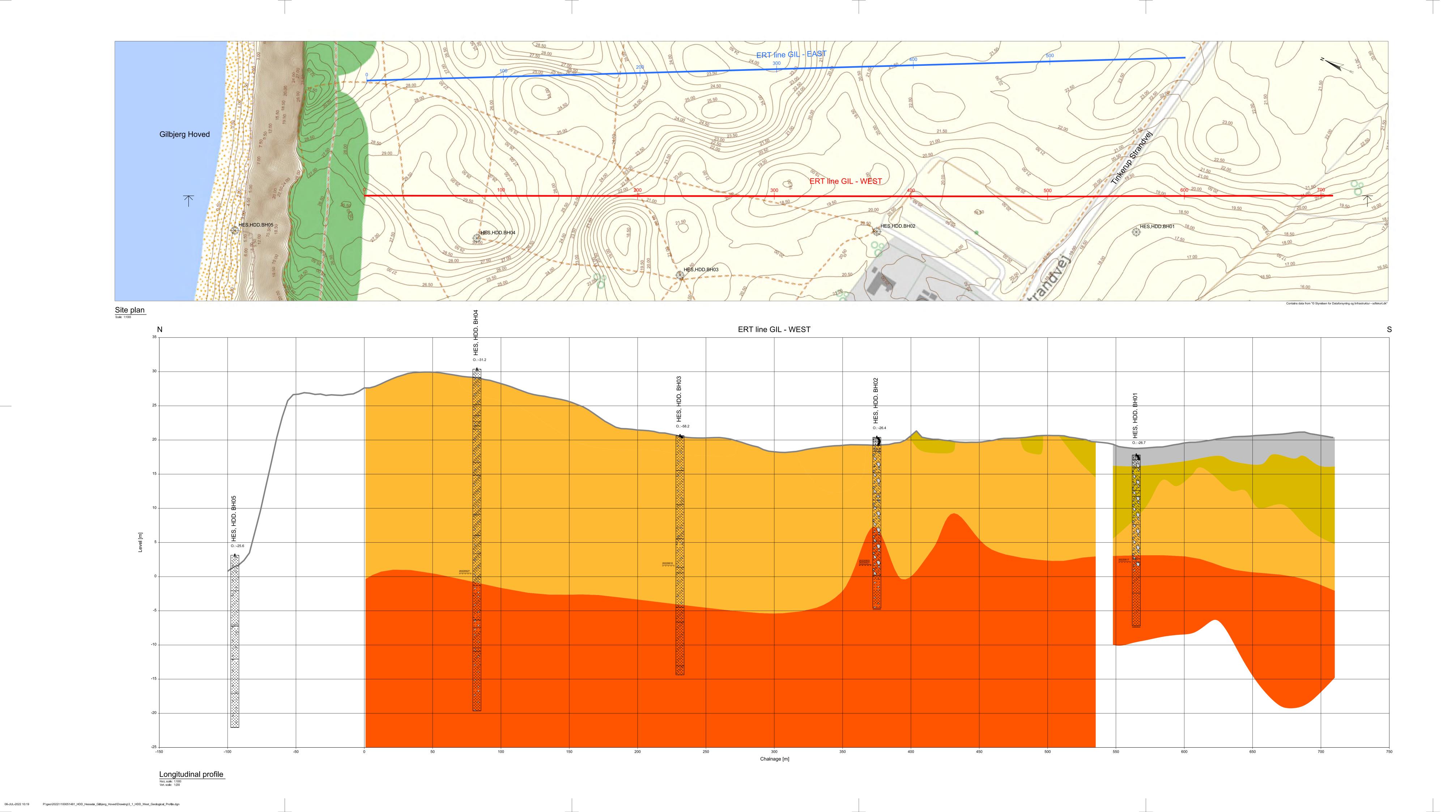








2.2 1



Cordana data from % Syruteen for Coaleteryaing og Infrastruktur - softworder

Legend:

Geotechnical borehole
Resistivity line (ERT) GIL - East
Resistivity line (ERT) GIL - West
Ground level (Ground level is created by topographic map, DHM/Terreen 0.5m grid.

Styrelsen for Dataforsyning og Infrastruktur - SDFP)

Coordinate system: ETRS89 UTM32N, Elevation DVR90

Format: 594x1260

Rev. Date
Signature Checked Approved
2022-07-06 JMM SHE RMH

Project no. 1100051481 Scale 1:1000; 1:200

Gilbjerg Hoved. HDD GIL - West.

HESSELØ OFFSHORE WINDFARM

Onshore DK. HDD site investigations

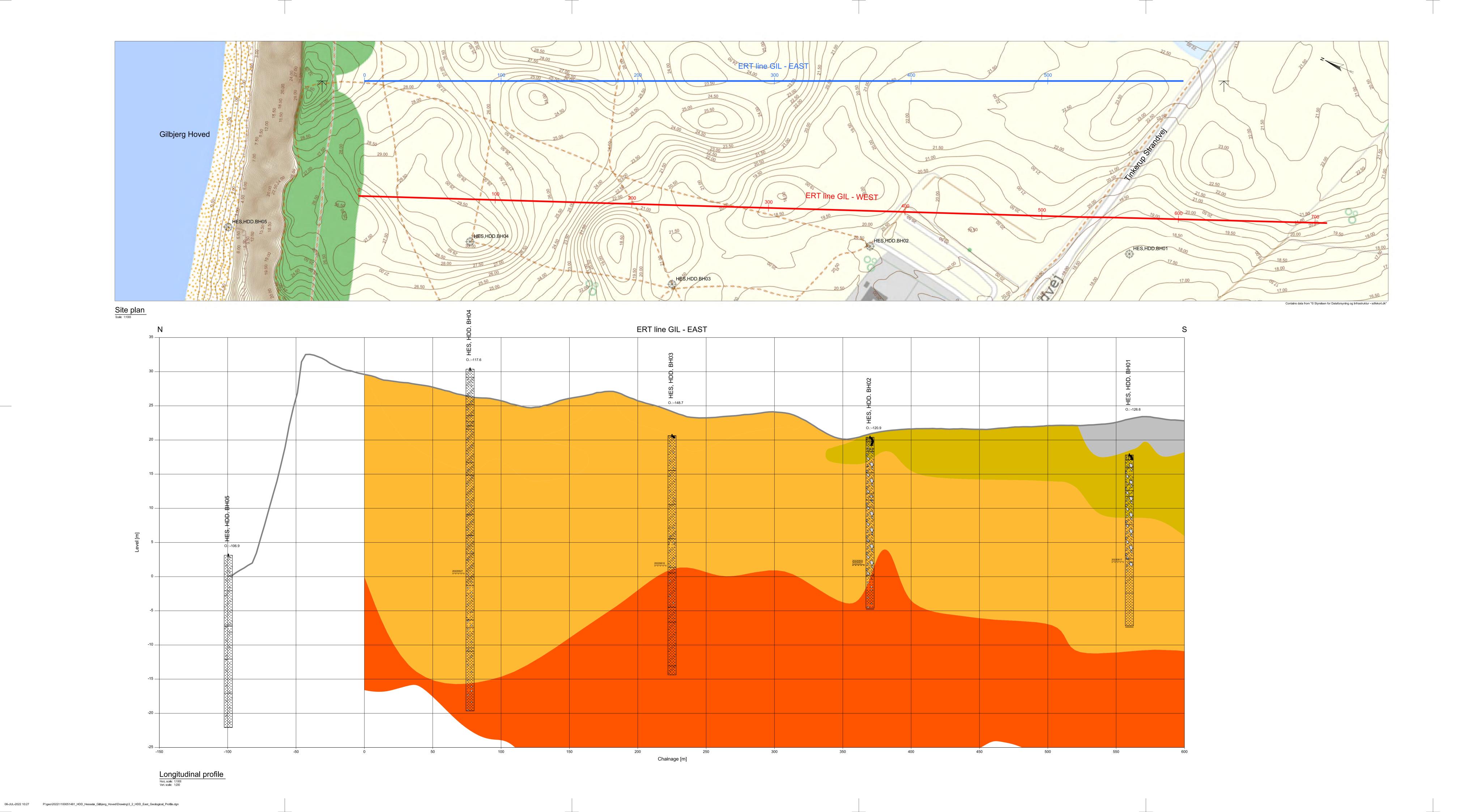
Geotechnical and geophysical longitudinal profile

Hannemanns Allé 53 DK-2300 København S Tlf. +45 5161 1000 Fax +45 5161 1001 www.ramboll.com

ENERGINET www.energinet.dk

Drawing no. Rev.

3.1 0



Geotechnical borehole Resistivity line (ERT) GIL - East Resistivity line (ERT) GIL - West Ground level (Ground level is created by topographic map, DHM/Terræn 0.5m grid. "Styrelsen for Dataforsyning og Infrastruktur - SDFI") Unsaturated Sand/Gravel Coordinate system: ETRS89 UTM32N, Elevation DVR90 Format: 594x1260 RAMBOLL 2022-07-06 JMN MSHE RMH Hannemanns Allé 53 DK-2300 København S Tlf. +45 5161 1000 Fax +45 5161 1001 www.ramboll.com Project no. 1100051481 Scale 1:1000; 1:200

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Drawing no. Rev.

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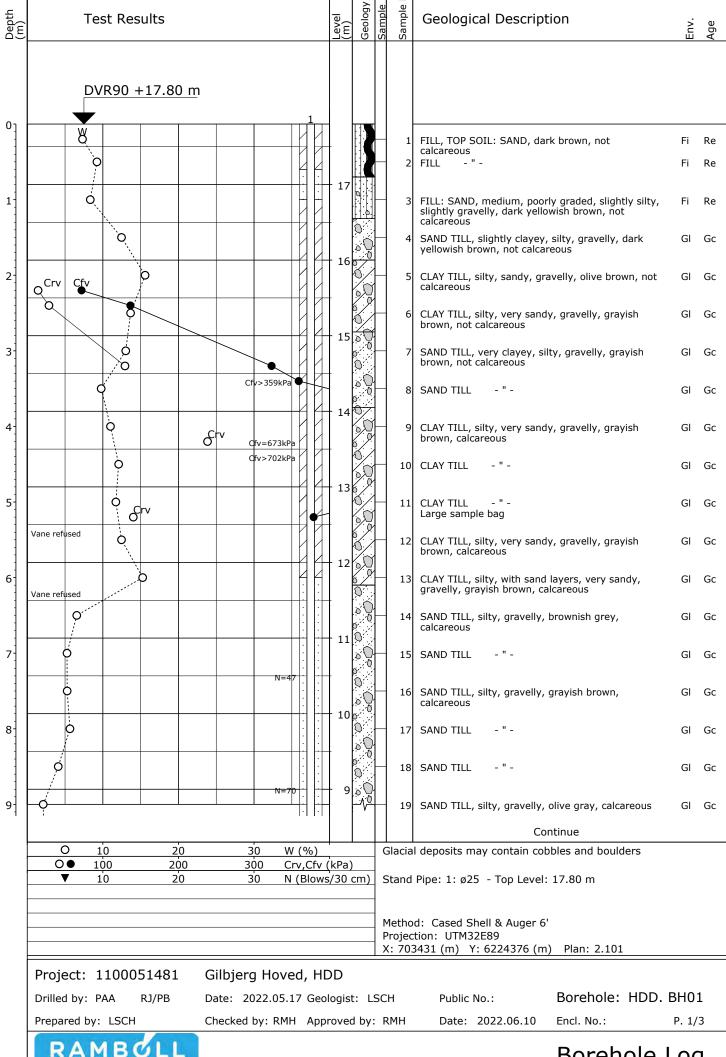
HESSELØ OFFSHORE WINDFARM

Onshore DK. HDD site investigations

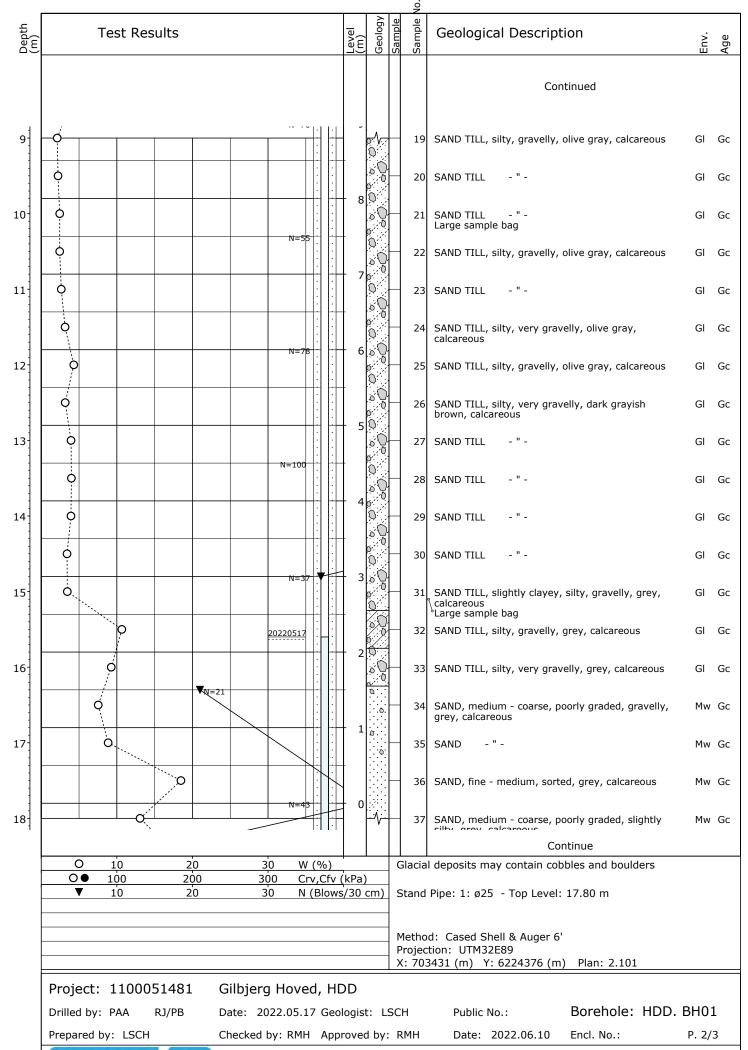
Geotechnical and geophysical longitudinal profile

Gilbjerg Hoved. HDD GIL - East.

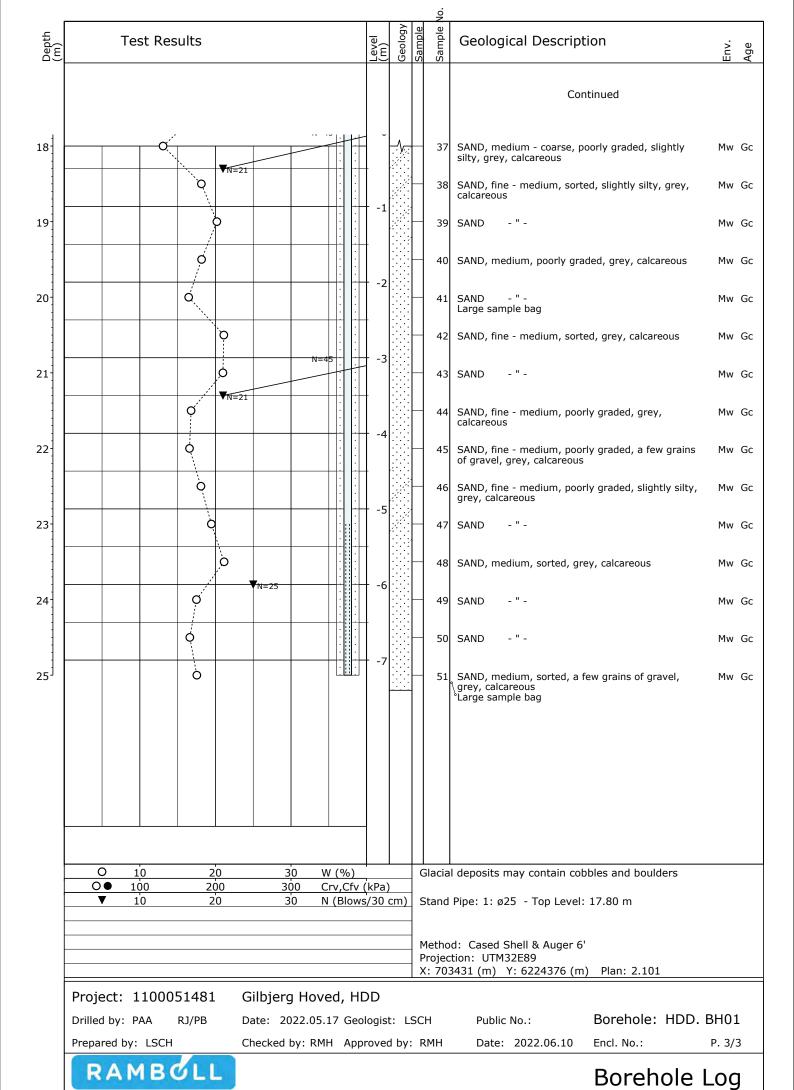
APPENDIX 4.1-4.5 - BOREHOLE LOGS

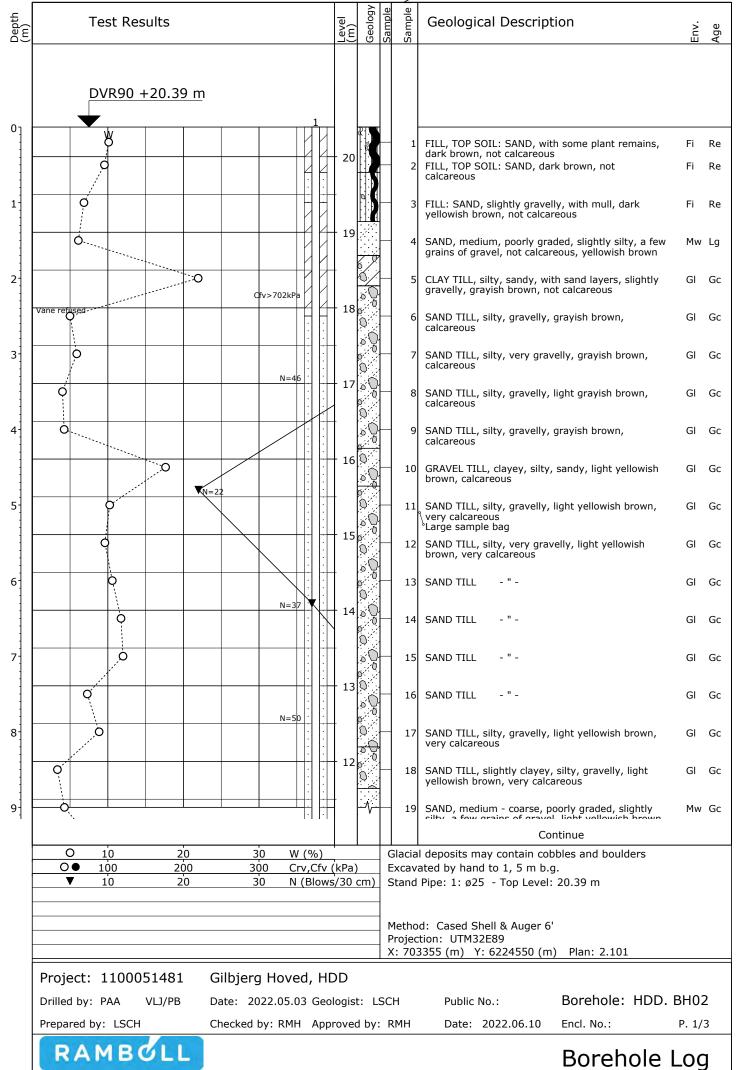


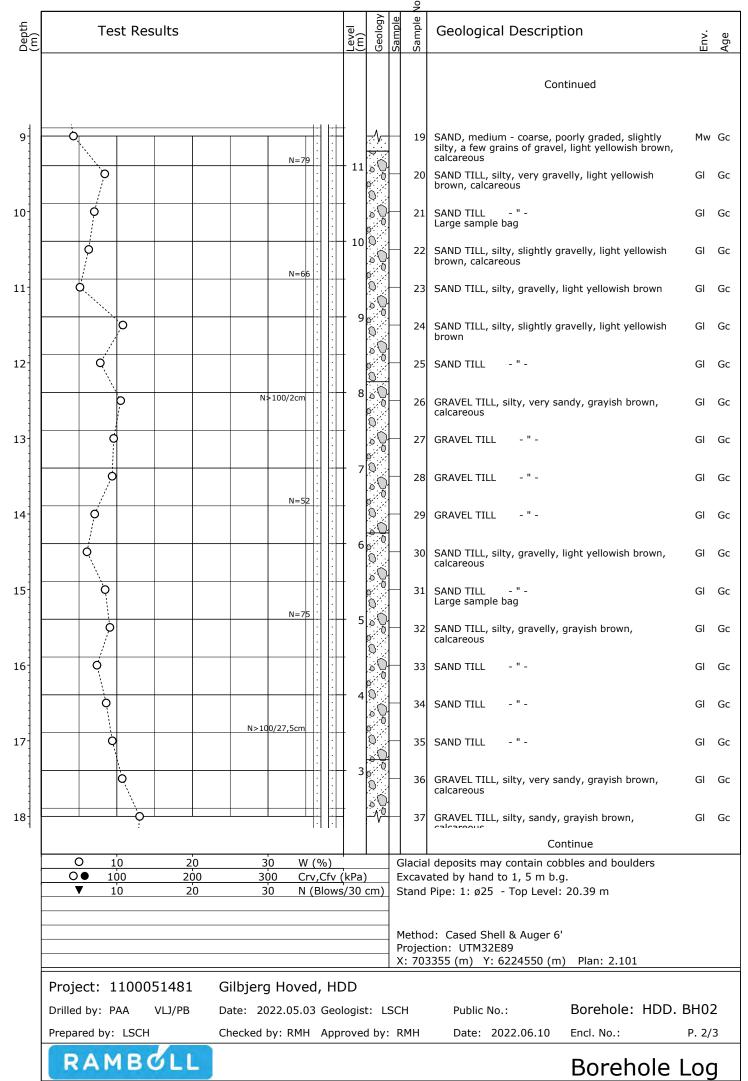
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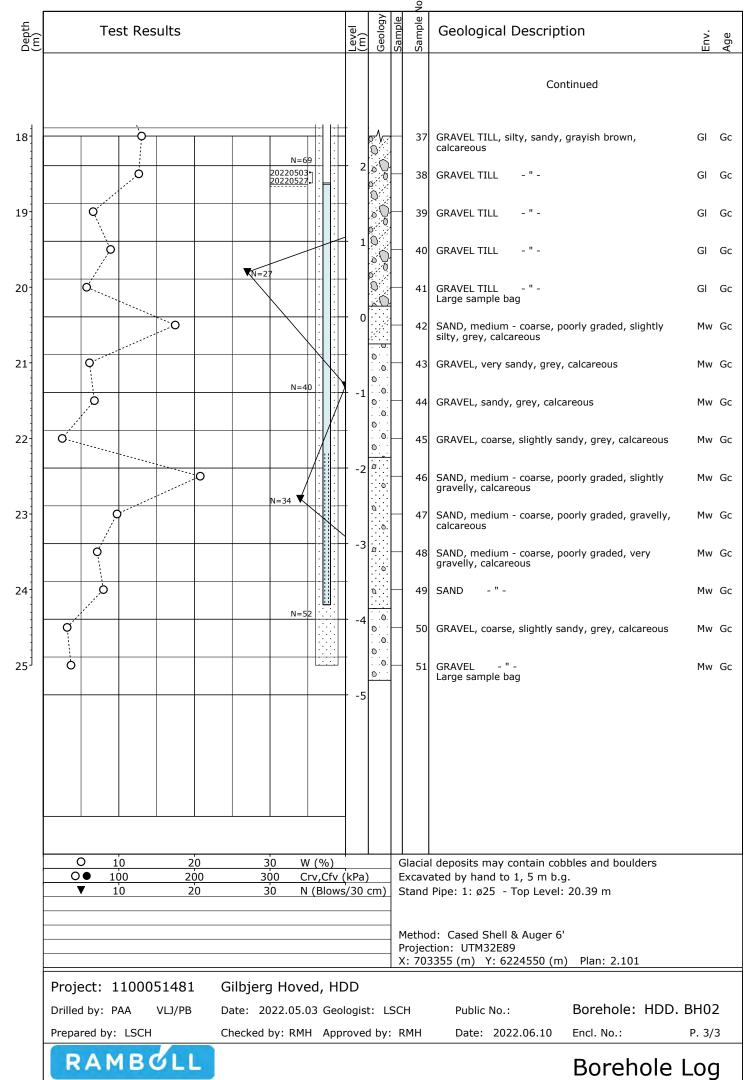


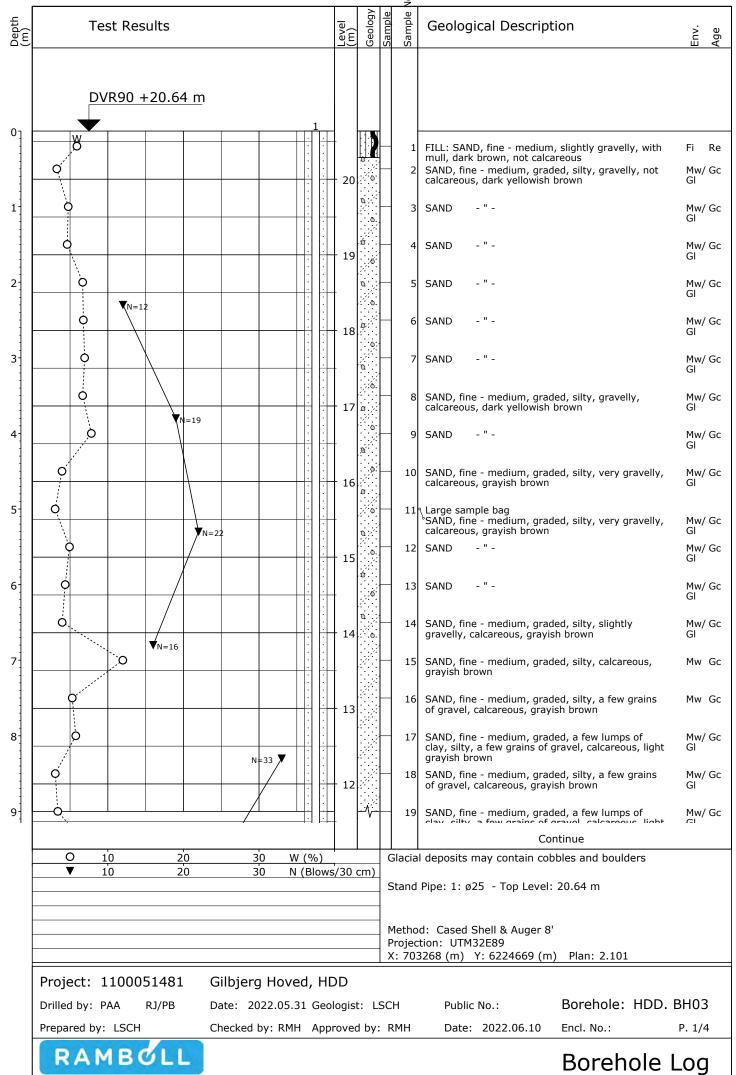
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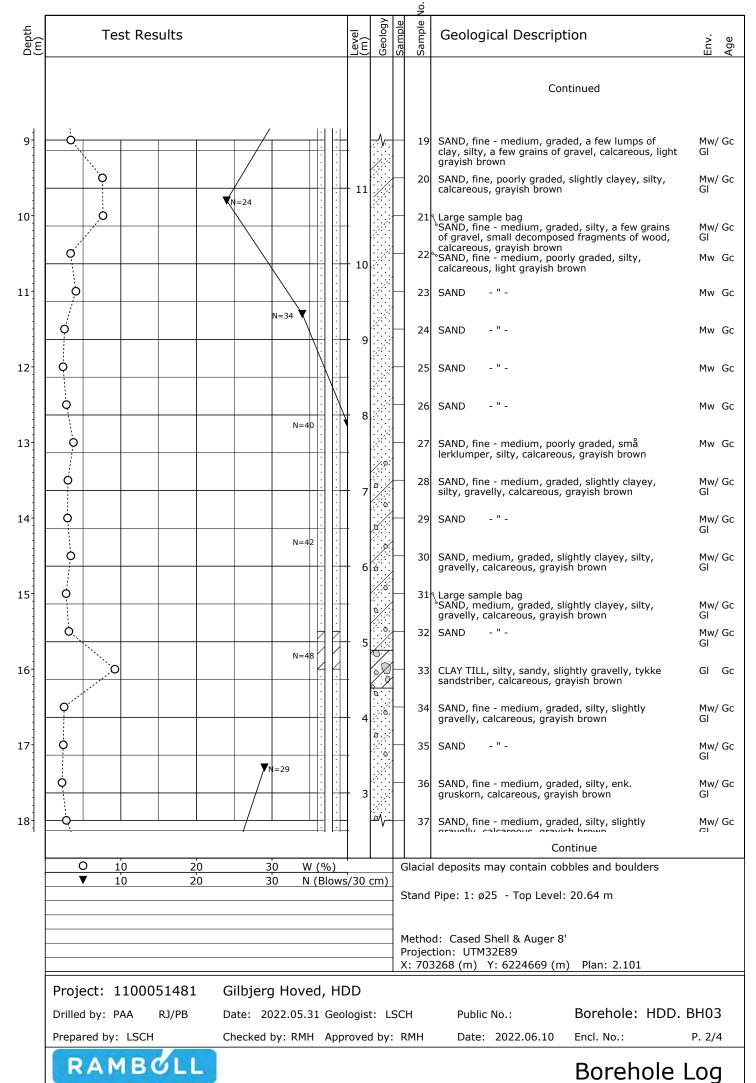


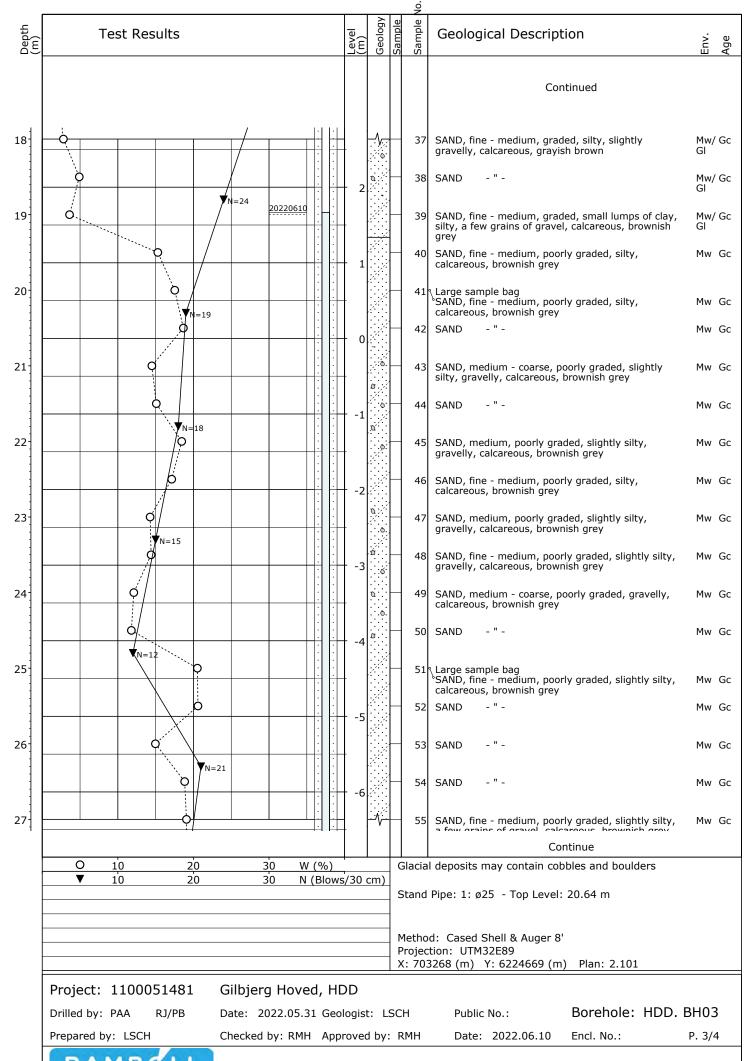




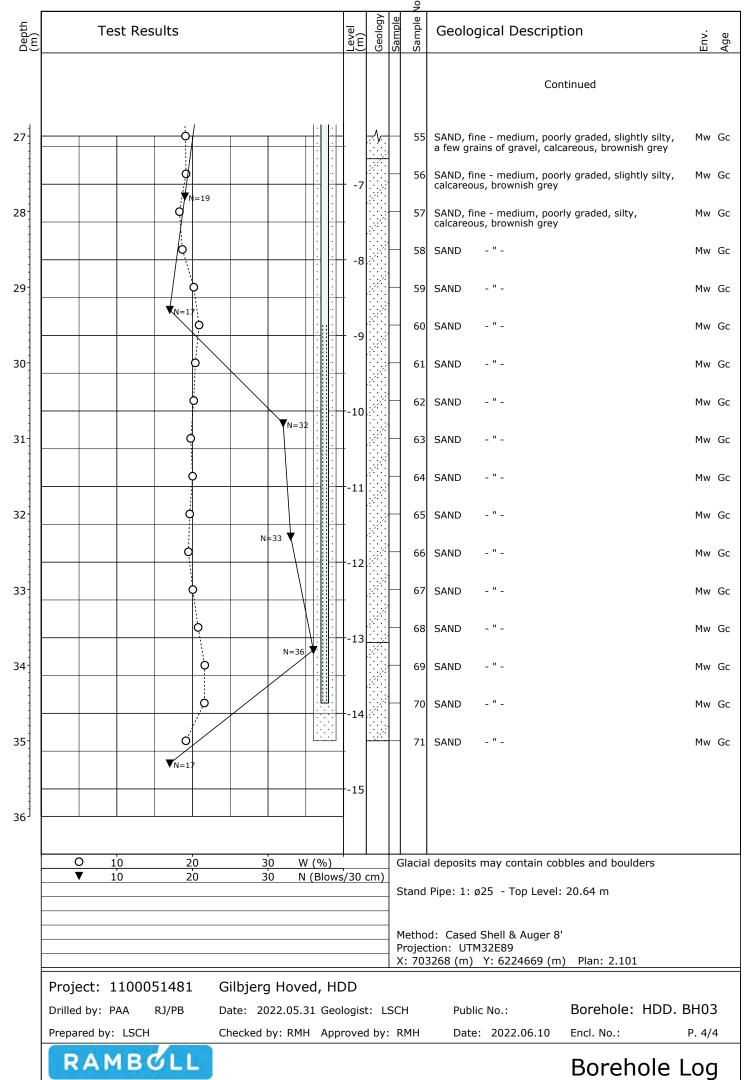


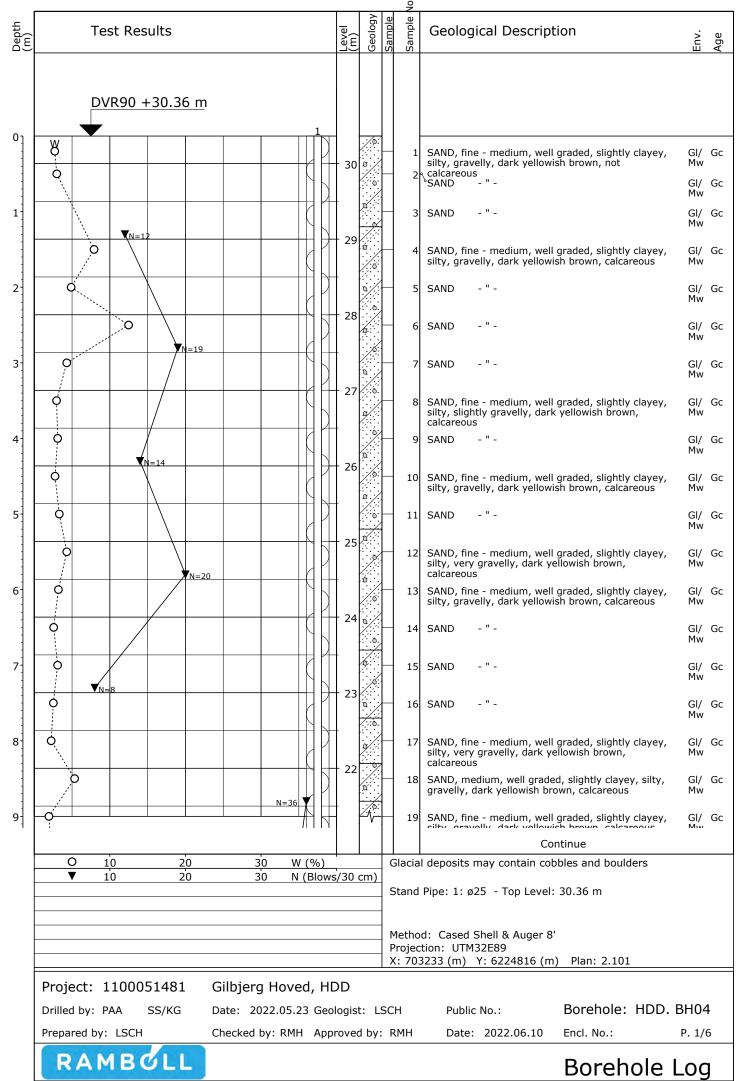


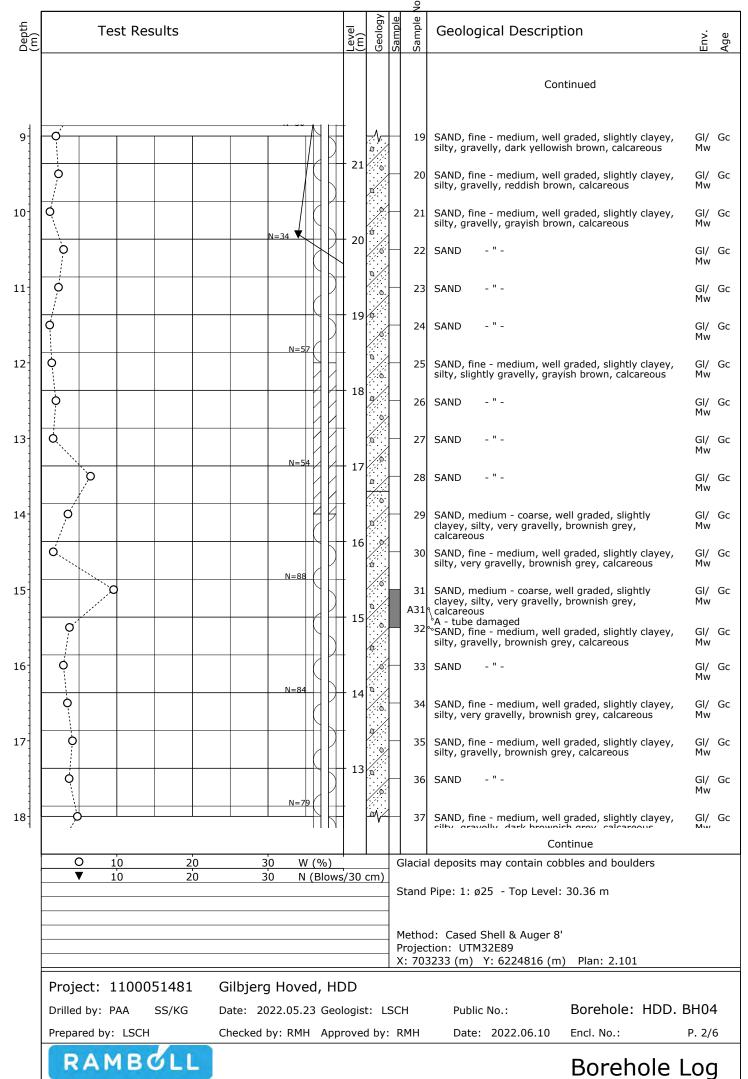


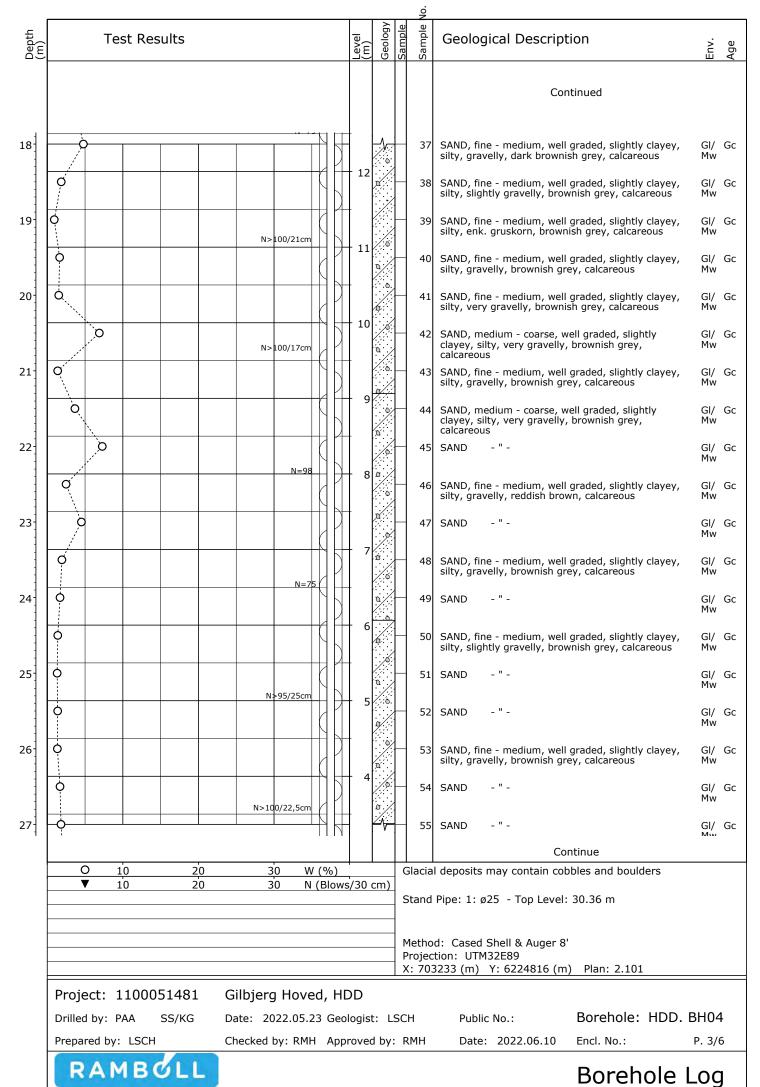


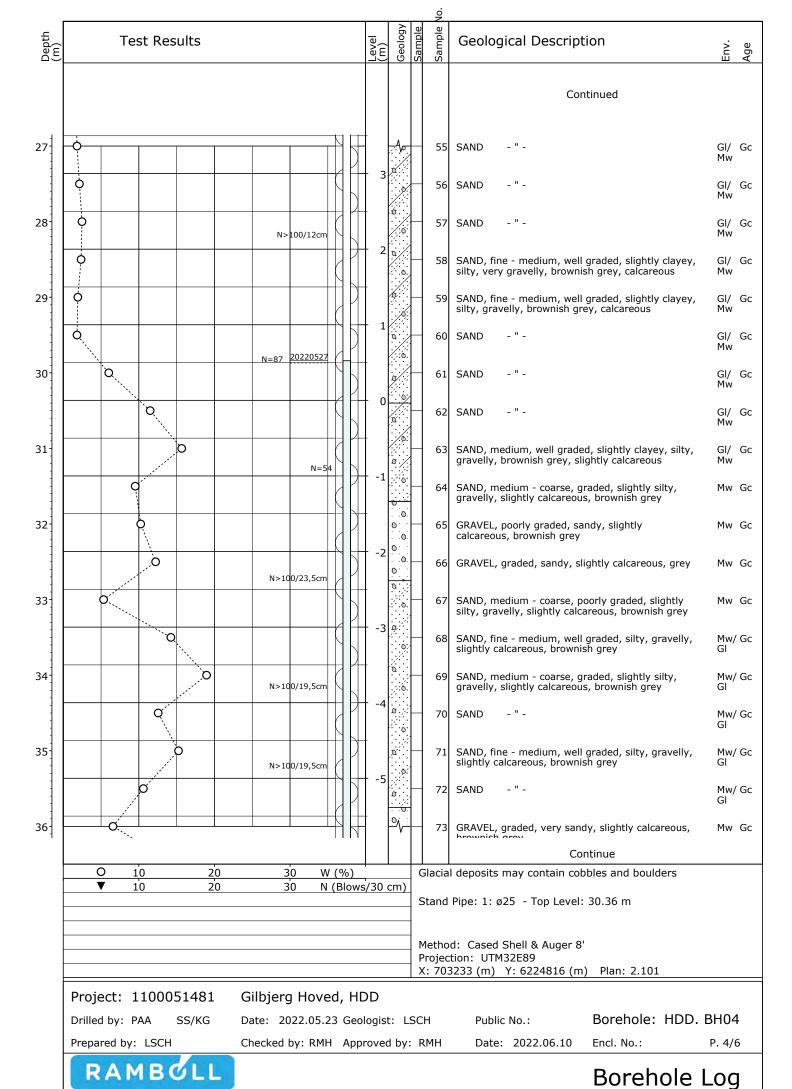
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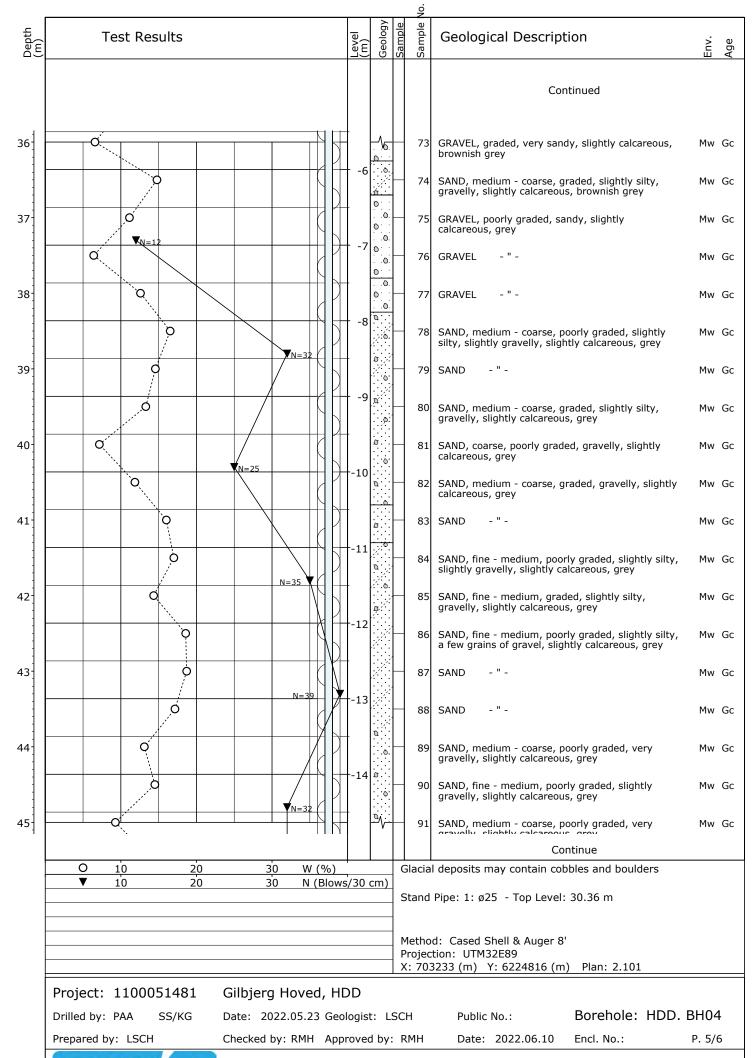




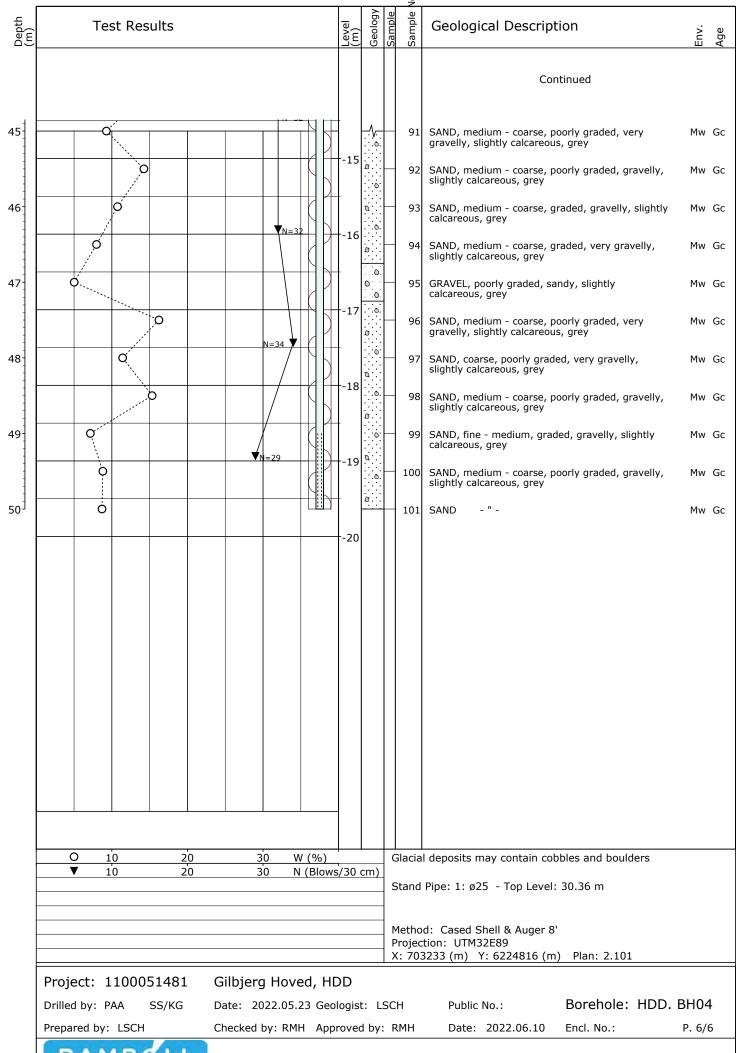




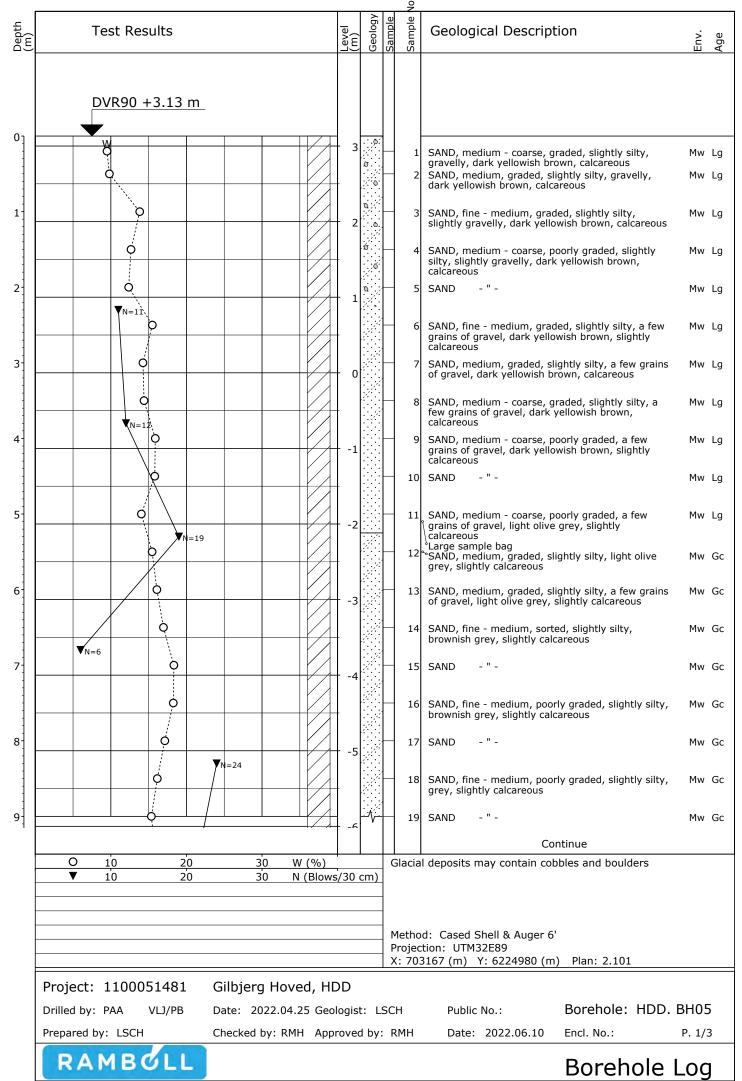


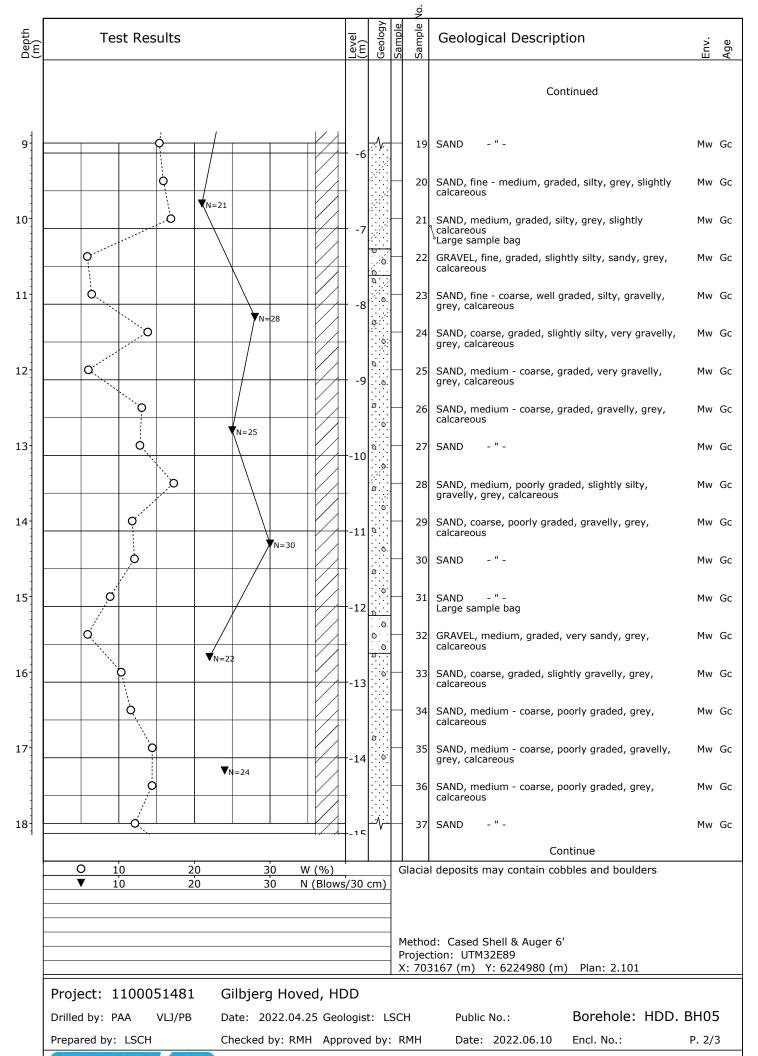


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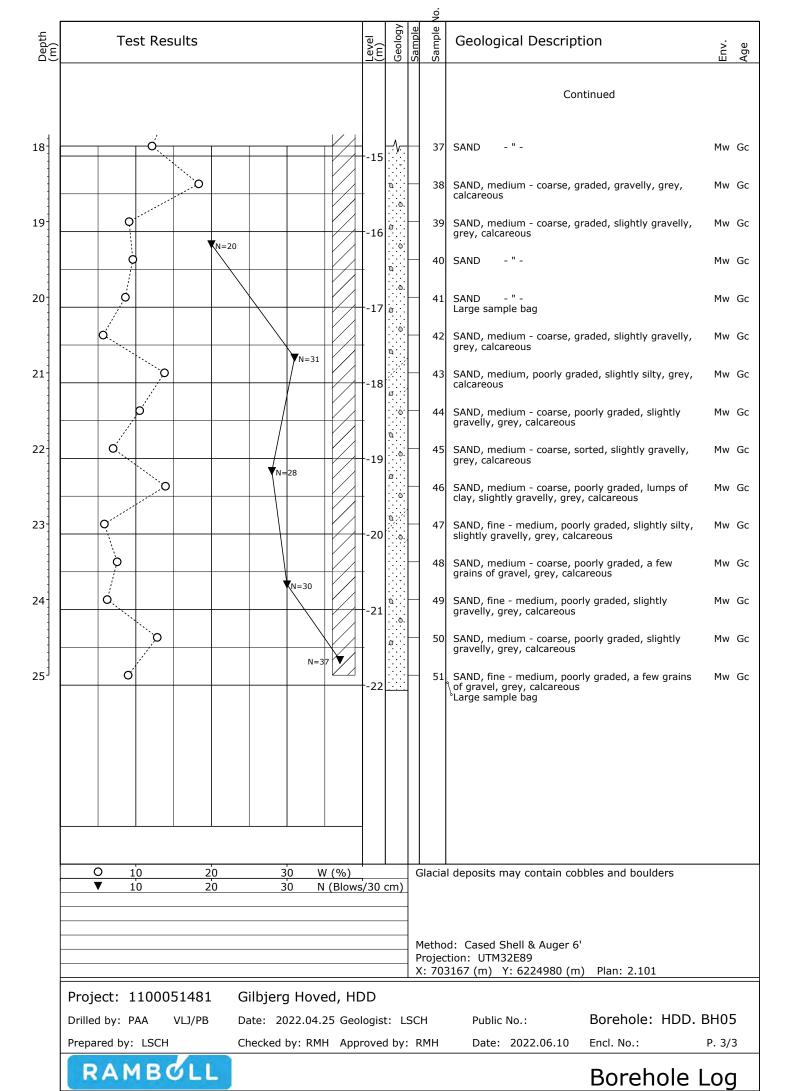


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APPENDIX 5.1-5.5 - PARTICLE SIZE DISTRIBUTION

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



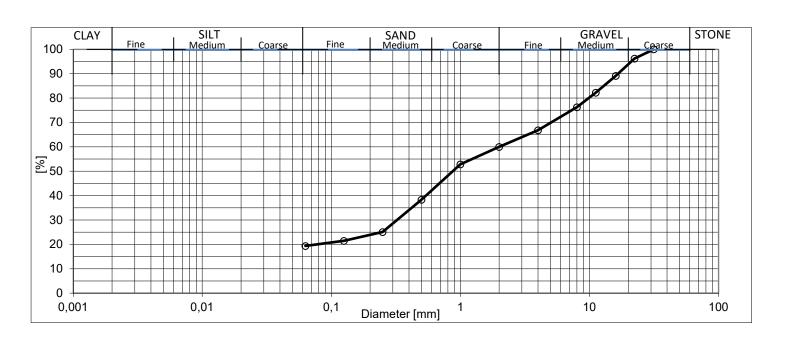
Inspection Section	BH01-S17-8,0
Sample Description	Sand, fine-medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	16. jun 2022

Siev	/ing
Sieve size	Passing
[mm]	%
31,5	100,0
22,4	96,2
16	89,1
11,2	82,2
8	76,3
4	66,8
2	60,0
1	52,8
0,5	38,3
0,25	25,0
0,125	21,4
0,063	19,4

Results	
D ₈₅	12,93
D ₆₀	2,00
D ₅₀	0,87
D ₁₅	
D ₁₀	

Coef	ficient of uniformity
D_{60}/D_{10}	



Made By: THAND

Checked By: CVZ

Date: 17. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



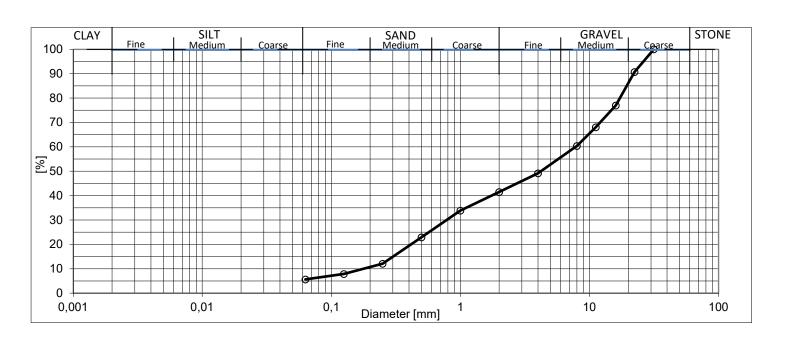
Inspection Section	BH01-S28-13,5
Sample Description	Sand, fine - medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	14. jun 2022

Siev	ring
Sieve size	Passing
[mm]	%
31,5	100,0
22,4	90,7
16	76,9
11,2	68,0
8	60,3
4	49,1
2	41,5
1	33,9
0,5	22,9
0,25	12,1
0,125	7,9
0,063	5,7

Results	
D ₈₅	19,49
D ₆₀	7,83
D ₅₀	4,22
D ₁₅	0,30
D ₁₀	0,18

Coefficient of uniformity	
D_{60}/D_{10}	44,12



Made By: MALT / THAND

Date: 14-06 / 16-06-2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



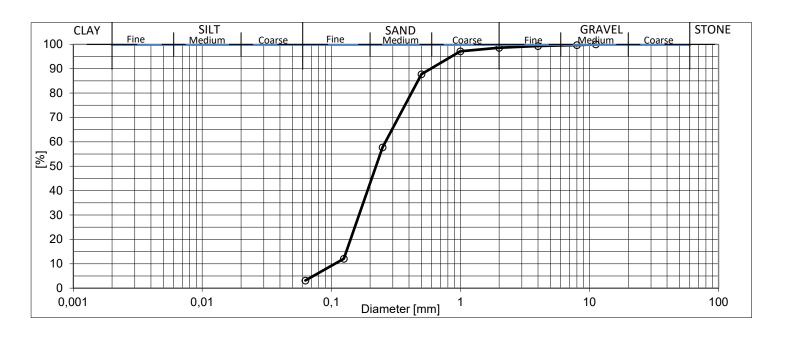
Inspection Section	BH01-S41-20
Sample Description	Sand, medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	20. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
11,2	100,0
8	99,7
4	99,3
2	98,6
1	97,1
0,5	87,8
0,25	57,7
0,125	12,1
0,063	3,2

Results	
D ₈₅	0,47
D ₆₀	0,26
D ₅₀	0,22
D ₁₅	0,13
D ₁₀	0,11

Coefficient of uniformity	
D_{60}/D_{10}	2,48



Made By: __THAND

Checked By: CVZ

Date: 20. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



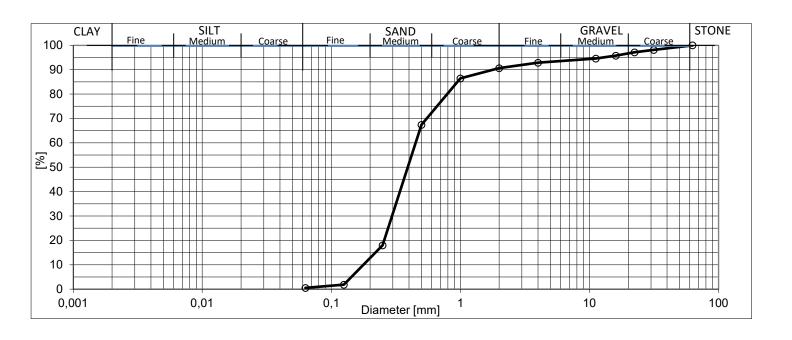
Inspection Section	BH01-S51-25
Sample Description	Sand, medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	20. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
63	100,0
31,5	98,1
22,4	97,1
16	95,7
11,2	94,6
4	92,9
2	90,6
1	86,4
0,5	67,4
0,25	18,0
0,125	1,8
0,063	0,5

Results	
D ₈₅	0,95
D ₆₀	0,45
D ₅₀	0,39
D ₁₅	0,22
D ₁₀	0,18

Coefficient of uniformity	
D_{60}/D_{10}	2,54



Made By: THAND

Checked By: CVZ

Date: 20. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



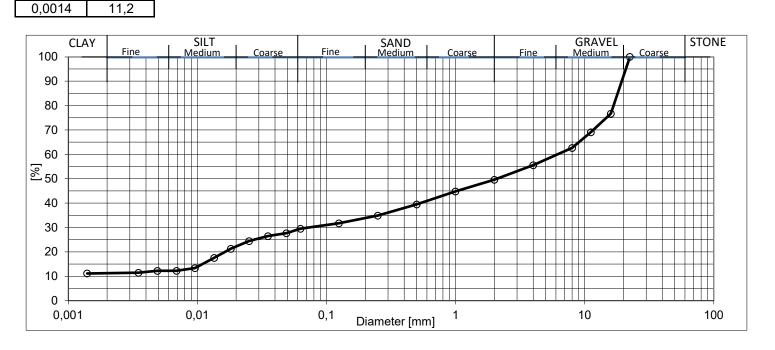
Inspection Section	BH02-S11-5
Sample Description	Sand, fine - medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	17. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
22,4	100,0
16	76,7
11,2	69,1
8	62,7
4	55,6
2	49,6
1	44,8
0,5	39,5
0,25	34,9
0,125	31,7
0,063	29,6
0,049	27,7
0,0353	26,5
0,0252	24,5
0,0182	21,3
0,0135	17,6
0,0096	13,4
0,0069	12,3
0,0049	12,3
0,0035	11,5
0.0044	440

Results	
D ₈₅	18,03
D ₆₀	6,16
D ₅₀	2,09
D ₁₅	0,012
D ₁₀	

Coefficient of uniformity		
D_{60}/D_{10}		



 Made By:
 THME
 Checked By:
 CVZ

 Date:
 17. jun 2022
 Date:
 21. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



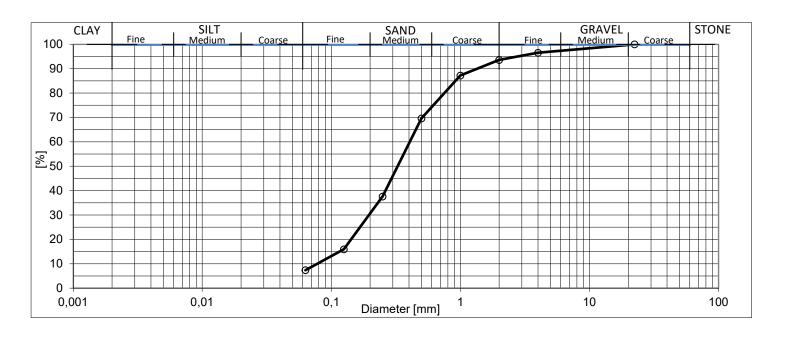
Inspection Section	BH02-S19-9
Sample Description	Sand, medium -coarse
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	16. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
22,4	100,0
4	96,5
2	93,6
1	87,2
0,5	69,6
0,25	37,6
0,125	15,9
0,063	7,4

Results	
D ₈₅	0,92
D ₆₀	0,41
D ₅₀	0,33
D ₁₅	0,12
D ₁₀	0,08

Coef	ficient of uniformity
D_{60}/D_{10}	5,23



Made By: THME

Date: 17. jun 2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



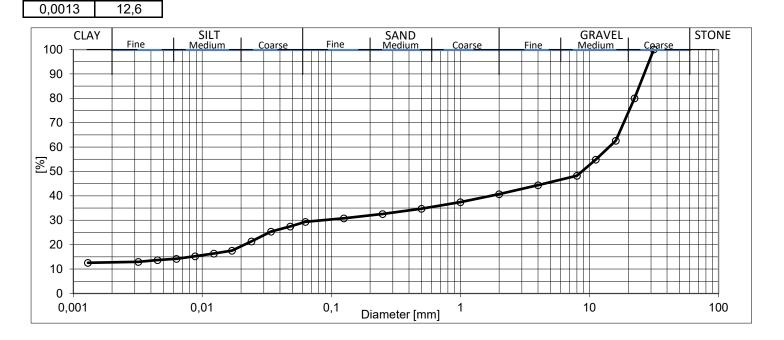
Inspection Section	BH02-S29-14
Sample Description	Gravel
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	17. jun 2022

0:	
Siev	
Sieve size	Passing
[mm]	%
31,5	100,0
22,4	80,0
16	62,6
11,2	54,9
8	48,3
4	44,4
2	40,7
1	37,4
0,5	34,8
0,25	32,6
0,125	30,8
0,063	29,4
0,0481	27,5
0,0341	25,4
0,0241	21,4
0,017	17,6
0,0123	16,4
0,0088	15,3
0,0063	14,2
0,0045	13,7
0,0032	13,0

Results	
D ₈₅	24,39
D ₆₀	14,19
D ₅₀	8,74
D ₁₅	0,008
D ₁₀	

Coefficient of uniformity	
D_{60}/D_{10}	



Made By: THME

Checked By: CVZ

Date: 17. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



Inspection Section	BH02-S43-21
Sample Description	Gravel, sandy
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	16. jun 2022

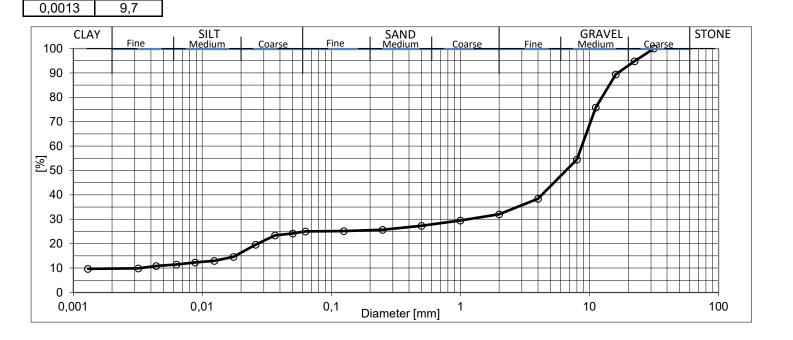
Location		
Sieving		
Sieve size	Passing	
[mm]	%	
31,5	100,0	
22,4	94,8	
16	89,4	
11,2	75,8	
8	54,5	
4	38,4	
2	32,0	
1	29,5	
0,5	27,4	
0,25	25,7	
0,125	25,2	
0,063	25,0	
0,0503	24,2	
0,0366	23,4	
0,0259	19,6	
0,0175	14,6	
0,0124	13,0	
0,0088	12,3	
0,0063	11,5	
0,0044	10,9	

0,0032

9,9

Results	
D ₈₅	14,25
D ₆₀	8,72
D ₅₀	6,58
D ₁₅	0,02
D ₁₀	0,003

Coefficient of uniformity		
D_{60}/D_{10}		



Made By: THME

Checked By: CVZ

Date: 17. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



Inspection Section	BH02-S48-23,5
Sample Description	Sand, medium - coarse
Location	Gilbjerg Hoved

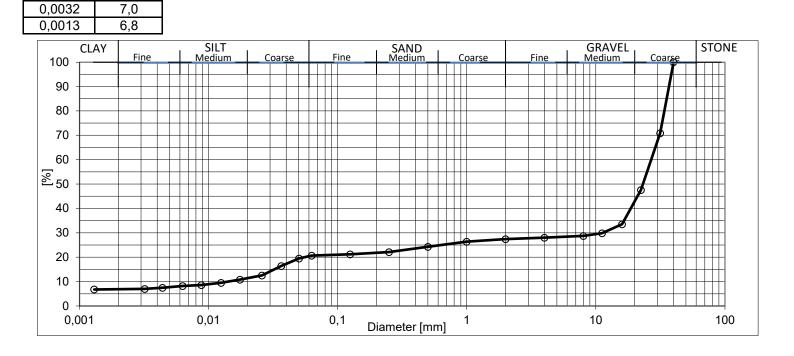
Lab Number	
Job Number	908091-1100051481
Date Sampled	16. jun 2022

Sieving		
Sieve size	Passing	
[mm]	%	
40	100,0	
31,5	70,8	
22,4	47,6	
16	33,6	
11,2	29,8	
8	28,7	
4	28,0	
2	27,4	
1	26,4	
0,5	24,3	
0,25	22,1	
0,125	21,2	
0,063	20,7	
0,0503	19,5	
0,0366	16,4	
0,0259	12,5	
0,0175	10,8	
0,0125	9,5	
0,0088	8,6	
0,0063	8,2	
0,0044	7,5	

7,0

Results		
D ₈₅	35,37	
D ₆₀	26,88	
D ₅₀	23,22	
D ₁₅	0,028	
D ₁₀	0,017	

Coefficient of uniformity		
D_{60}/D_{10}		



Made By: __THAND

Checked By: CVZ

Date: 17. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



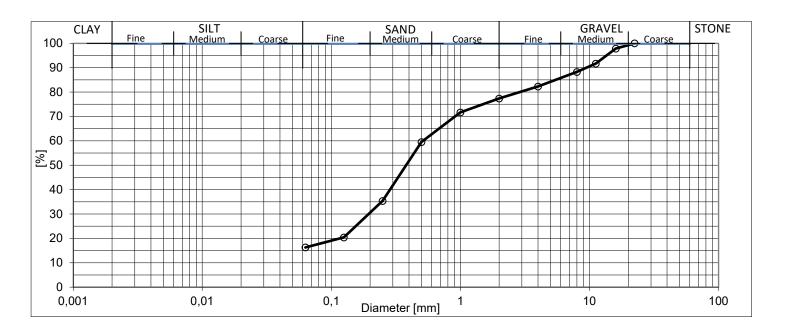
Inspection Section	BH03-S5-2
Sample Description	Sand, fine - medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
22,4	100,0
16	97,9
11,2	91,7
8	88,3
4	82,3
2	77,3
1	71,7
0,5	59,5
0,25	35,3
0,125	20,4
0,063	16,3

Results	
D ₈₅	5,49
D ₆₀	0,51
D ₅₀	0,38
D ₁₅	
D ₁₀	

Coefficient of uniformity		
D_{60}/D_{10}		•



Made By: THAND

Checked By: CVZ

Date: 16. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



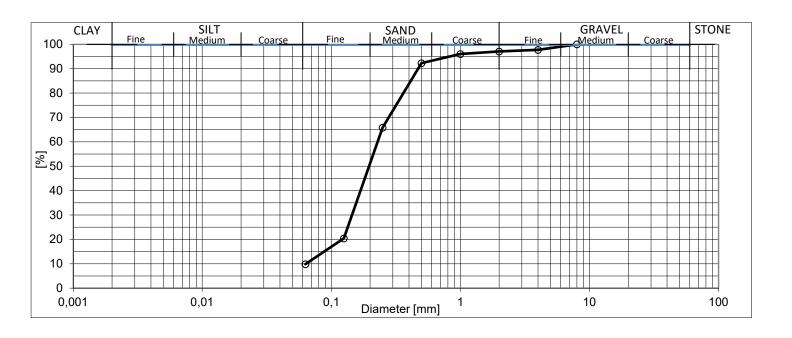
Inspection Section	BH03-S15-7
Sample Description	Sand, fine - medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
8	100,0
4	97,7
2	97,1
1	96,0
0,5	92,2
0,25	65,8
0,125	20,4
0,063	9,9

Results	
D ₈₅	0,41
D ₆₀	0,23
D ₅₀	0,20
D ₁₅	0,09
D ₁₀	0,06

Coefficient of uniformity	
D_{60}/D_{10}	3,60



Made By: THAND

Date: 15. jun 2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



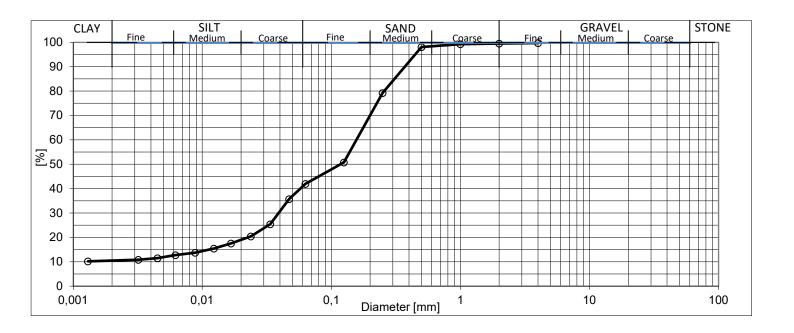
Inspection Section	BH03-S21-10m
Sample Description	Sand, Fine medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Siev	ring
Sieve size	Passing
[mm]	%
4	99,7
2	99,5
1	99,2
0,5	98,0
0,25	79,2
0,125	50,7
0,063	41,9
0,0471	35,7
0,0336	25,4
0,0238	20,4
0,0167	17,5
0,0123	15,4
0,0088	13,7
0,0062	12,7
0,0045	11,5
0,0032	10,8
0,0013	10,2

Results	
D ₈₅	0,31
D ₆₀	0,16
D ₅₀	0,12
D ₁₅	0,012
D ₁₀	

Coef	ficient of uniformity
D_{60}/D_{10}	



Made By: THAND

Date: 15. jun 2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



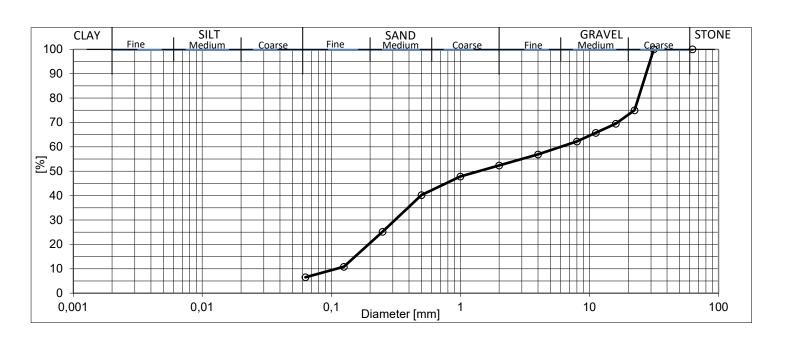
Inspection Section	BH03-S30-15
Sample Description	Sand, medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	16. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
63	100,0
31,5	100,0
22,4	75,0
16	69,5
11,2	65,8
8	62,2
4	56,9
2	52,4
1	47,8
0,5	40,2
0,25	25,2
0,125	10,8
0,063	6,5

Results		
D ₈₅	25,67	
D ₆₀	5,98	
D ₅₀	1,40	
D ₁₅	0,15	
D ₁₀	0,11	

Coefficient of uniformity	
D_{60}/D_{10}	54,63



Made By: THAND

Date: / 15-06-2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11

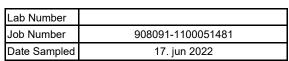


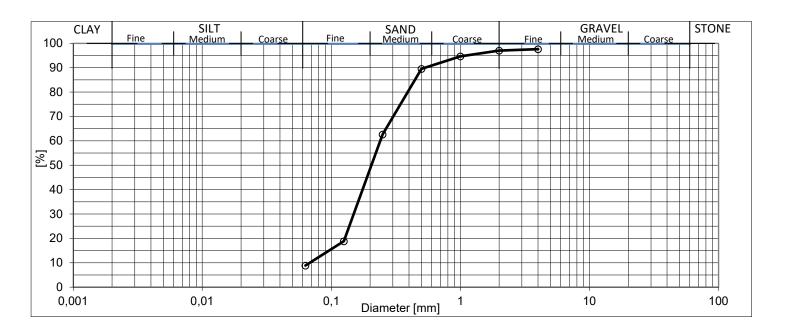
Inspection Section	BH03-S41-20
Sample Description	Sand, fine - medium
Location	Gilbjerg Hoved

_	0	
	Re	sults
	D_{85}	0,45
	D_{60}	0,24
	D ₅₀	0,20
	D ₁₅	0,10
	D ₁₀	0,07

Sieving	
Sieve size	Passing
[mm]	%
4	97,6
2	97,0
1	94,7
0,5	89,5
0,25	62,6
0,125	18,8
0,063	8,9

Coef	ficient of uniformity
D_{60}/D_{10}	3,53





Made By: __THAND

Date: 17. jun 2022

Checked By: CVZ

Date: ___17. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



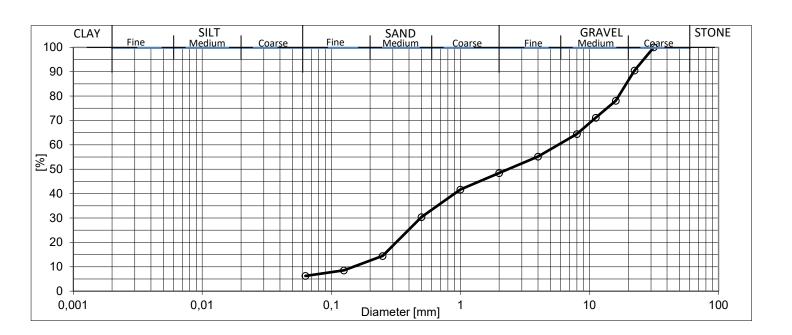
Inspection Section	BH03-S51-25
Sample Description	Sand, medium - coarse
Location	Gilbjerg Hoved

5	Lab Number	
m - coarse	Job Number	908091-1100051481
ed	Date Sampled	14. jun 2022
Results		

Sieving	
Sieve size	Passing
[mm]	%
31,5	100,0
22,4	90,5
16	78,1
11,2	71,1
8	64,4
4	55,2
2	48,4
1	41,7
0,5	30,4
0,25	14,4
0,125	8,5
0,063	6,3

Results	
D ₈₅	19,30
D ₆₀	5,74
D ₅₀	2,35
D ₁₅	0,26
D ₁₀	0,15

Coefficient of uniformity		
D_{60}/D_{10}	38,59	



Made By: MALT / THAND

Date: 14-06 / 16-06-2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



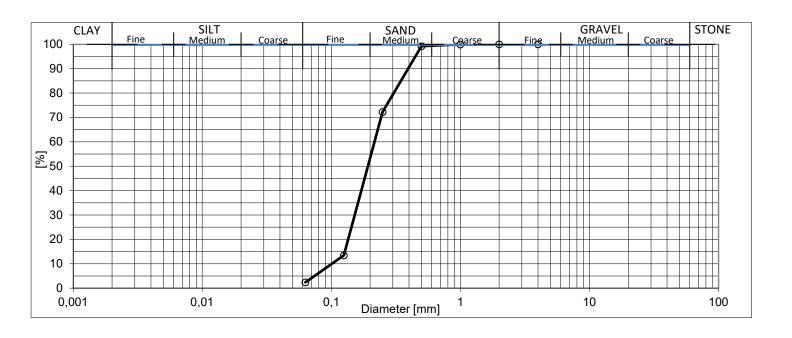
Inspection Section	BH03-S63-31,5
Sample Description	Sand, Fine medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
4	100,0
2	100,0
1	99,9
0,5	99,2
0,25	72,2
0,125	13,5
0,063	2,4

Results	
D ₈₅	0,35
D ₆₀	0,22
D ₅₀	0,19
D ₁₅	0,13
D ₁₀	0,10

Coefficient of uniformity	
D_{60}/D_{10}	2,15



Made By: __THAND

Checked By: CVZ

Date: 15. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



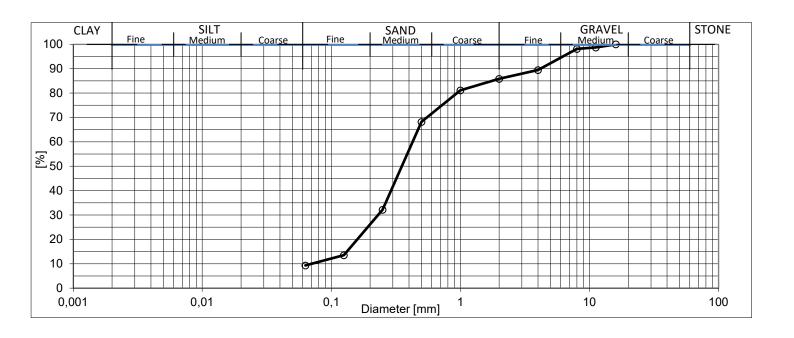
Inspection Section	BH04-S5-2
Sample Description	Sand, fine - medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Siev	/ina
Sieve size	Passing
[mm]	%
16	100,0
11,2	98,7
8	98,1
4	89,5
2	85,8
1	81,1
0,5	68,2
0,25	32,1
0,125	13,5
0,063	9,3

Results	
D ₈₅	1,77
D ₆₀	0,43
D ₅₀	0,35
D ₁₅	0,13
D ₁₀	0,07

Coefficient of uniformity	
D_{60}/D_{10}	6,10



Made By: THAND

Date: 15. jun 2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



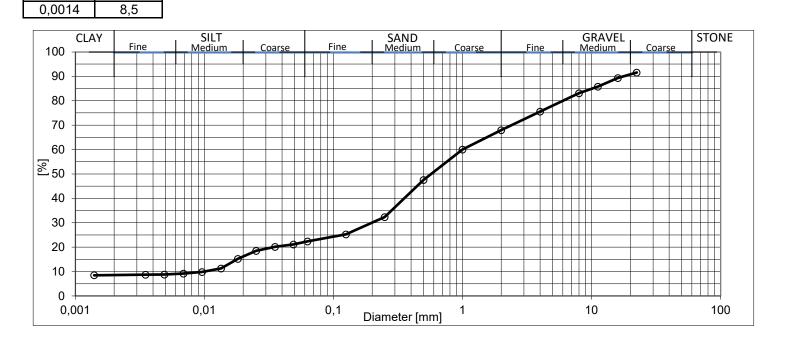
Inspection Section	BH04-S18-8,5
Sample Description	Sand, medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Siev	ring
Sieve size	Passing
[mm]	%
22,4	91,6
16	89,3
11,2	85,8
8	83,1
4	75,5
2	68,0
1	60,0
0,5	47,6
0,25	32,3
0,125	25,3
0,063	22,3
0,049	21,1
0,0353	20,1
0,0252	18,5
0,0182	15,2
0,0135	11,3
0,0096	9,8
0,0069	9,2
0,0049	8,8
0.0035	8.7

Results	
D ₈₅	10,17
D ₆₀	1,00
D ₅₀	0,57
D ₁₅	0,018
D ₁₀	0,010

Coefficient of uniformity	
D_{60}/D_{10}	99,96



Made By: THAND

Checked By: CVZ

Date: 15. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



Inspection Section	BH04-S29-14
Sample Description	Sand, medium - coarse
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

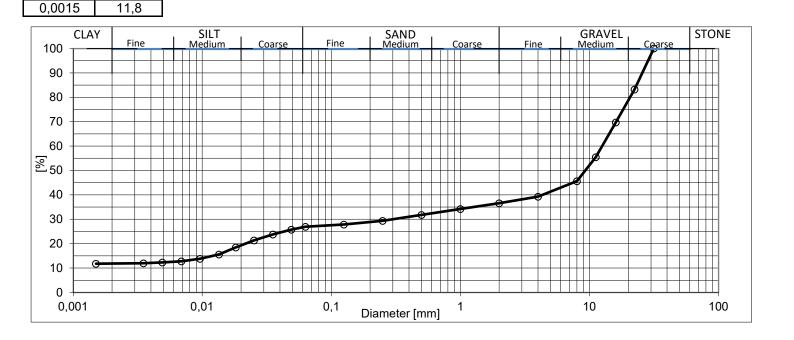
1	
Siev	/ing
Sieve size	Passing
[mm]	%
31,5	100,0
22,4	83,2
16	69,7
11,2	55,5
8	45,7
4	39,3
2	36,6
1	34,3
0,5	31,8
0,25	29,4
0,125	27,8
0,063	26,9
0,049	25,8
0,0353	23,8
0,0252	21,4
0,0182	18,5
0,0135	15,6
0,0096	13,8
0,0069	12,8
0,0049	12,3

0,0035

12,0

Results	
D ₈₅	23,24
D ₆₀	12,54
D ₅₀	9,28
D ₁₅	0,013
D ₁₀	

Coef	ficient of uniformity
D_{60}/D_{10}	



 Made By:
 THAND
 Checked By:
 CVZ

 Date:
 15. jun 2022
 Date:
 21. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



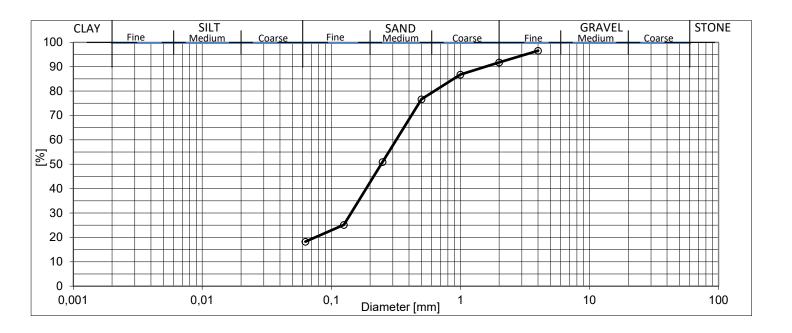
Inspection Section	BH04-S90-44,5
Sample Description	Sand, Fine medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Siev	/ing
Sieve size	Passing
[mm]	%
4	96,5
2	91,7
1	86,7
0,5	76,7
0,25	50,9
0,125	25,1
0,063	18,2

Re	sults
D ₈₅	0,89
D ₆₀	0,32
D ₅₀	0,24
D ₁₅	
D ₁₀	

Coefficient of uniformity	
D_{60}/D_{10}	



Made By: __THAND

Date: 15. jun 2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



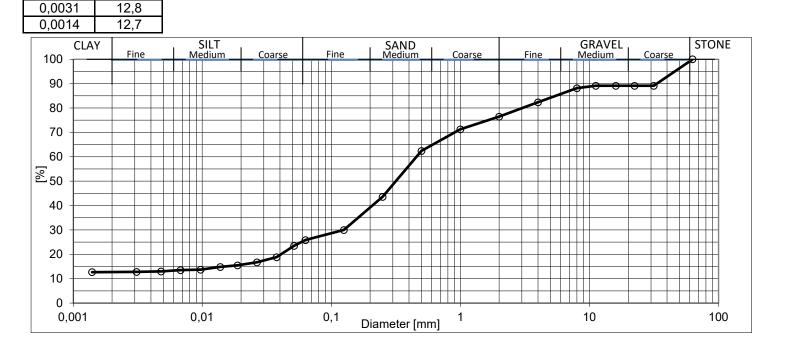
Inspection Section	BH04-S53-26
Sample Description	Sand, Fine medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Location	
Siev	⁄ing
Sieve size	Passing
[mm]	%
63	100,0
31,5	89,1
22,4	89,1
16	89,1
11,2	89,1
8	88,1
4	82,4
2	76,5
1	71,2
0,5	62,4
0,25	43,6
0,125	30,0
0,063	25,9
0,0514	23,5
0,0377	18,8
0,0265	16,7
0,0188	15,5
0,0138	14,8
0,0097	13,7
0,0068	13,5
0,0048	13,0

Results	
D ₈₅	5,49
D ₆₀	0,46
D ₅₀	0,32
D ₁₅	0,018
D ₁₀	

Coefficient of uniformity	
D_{60}/D_{10}	



Made By: __THAND

Checked By: CVZ

Date: 15. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



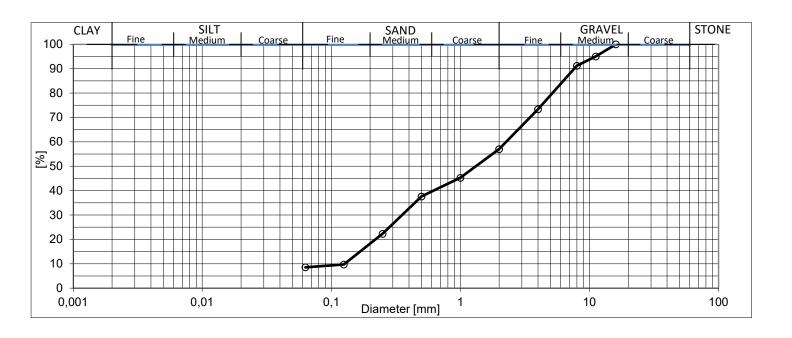
Inspection Section	BH04-S64-31,5
Sample Description	Sand, medium - coarse
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Sieving	
Passing	
%	
100,0	
95,0	
91,2	
73,4	
57,0	
45,2	
37,6	
22,3	
9,7	
8,6	

Results	
D ₈₅	6,29
D ₆₀	2,27
D ₅₀	1,32
D ₁₅	0,17
D ₁₀	0,13

Coefficient of uniformity	
D_{60}/D_{10}	17,87



Made By: THAND

Checked By: CVZ

Date: 15. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



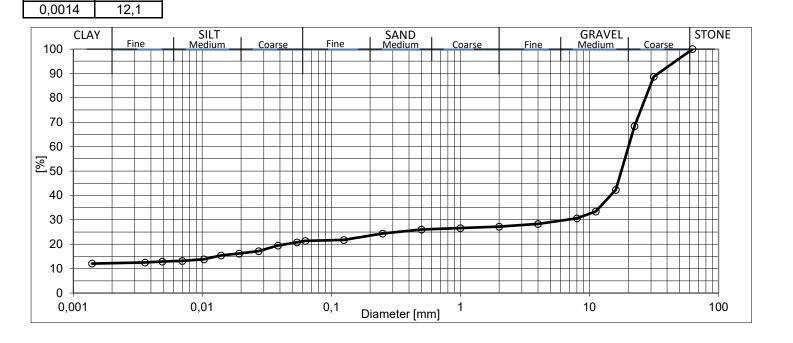
Inspection Section	BH04-S73-36
Sample Description	Gravel, sandy
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	16. jun 2022

Location	
Sieving	
Sieve size	Passing
[mm]	%
63	100,0
31,5	88,7
22,4	68,4
16	42,3
11,2	33,5
8	30,6
4	28,4
2	27,2
1	26,6
0,5	26,1
0,25	24,4
0,125	21,8
0,063	21,4
0,0541	20,8
0,0387	19,5
0,0274	17,2
0,0193	16,2
0,014	15,4
0,0103	13,8
0,007	13,2
0,0049	12,9
0,0036	12,5

Results	
D ₈₅	29,61
D ₆₀	20,10
D ₅₀	17,67
D ₁₅	0,013
D ₁₀	

Coefficient of uniformity	
D_{60}/D_{10}	



Made By: THAND

Checked By: CVZ

Date: 16. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



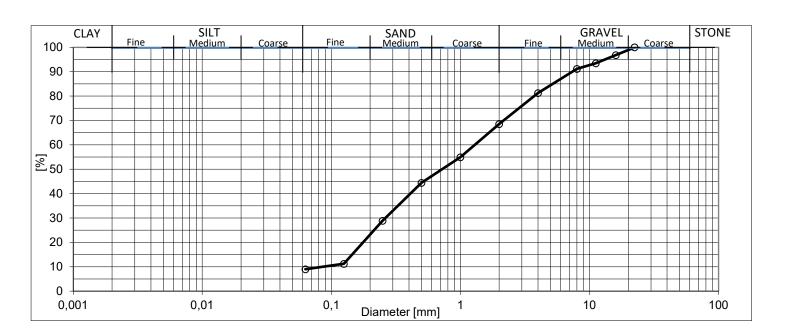
Inspection Section	BH04-S80-39,5
Sample Description	Sand, medium - coarse
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
22,4	100,0
16	96,8
11,2	93,5
8	91,2
4	81,2
2	68,5
1	54,9
0,5	44,4
0,25	28,9
0,125	11,2
0,063	9,0

Results	
D ₈₅	5,20
D ₆₀	1,30
D ₅₀	0,72
D ₁₅	0,14
D ₁₀	0,09

Coefficient of uniformity	
D_{60}/D_{10}	15,14



Made By: THAND

Checked By: CVZ

Date: 15. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



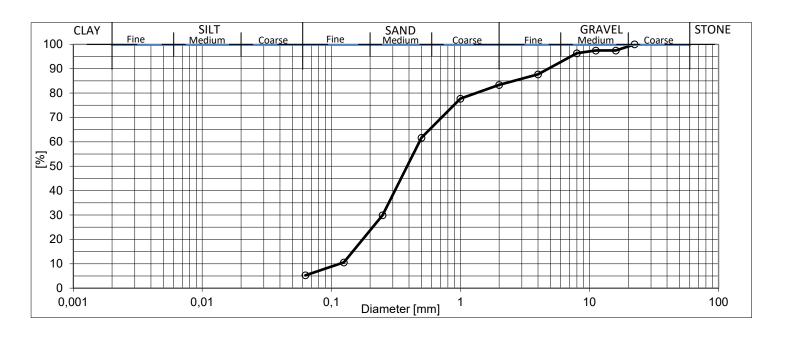
Inspection Section	BH04-S90-44,5
Sample Description	Sand, Fine medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
22,4	100,0
16	97,4
11,2	97,4
8	96,3
4	87,7
2	83,3
1	77,7
0,5	61,7
0,25	29,9
0,125	10,5
0,063	5,3

Results	
D ₈₅	2,61
D ₆₀	0,48
D ₅₀	0,39
D ₁₅	0,15
D ₁₀	0,12

Coefficient of uniformity	
D_{60}/D_{10}	4,13



Made By: THAND

Date: 15. jun 2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



Inspection Section	BH04-S95-47
Sample Description	Gravel
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	15. jun 2022

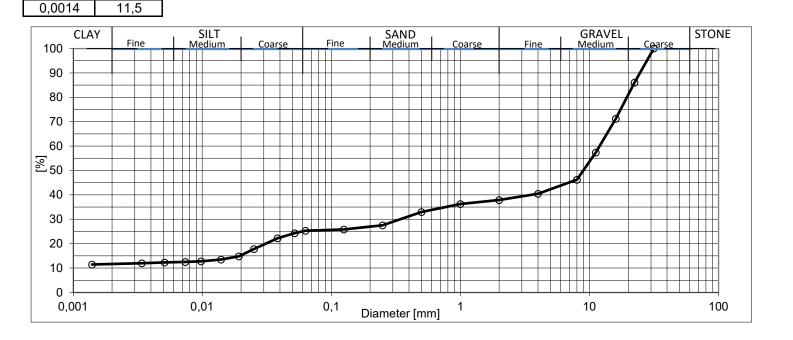
Sieving			
Sieve size	Passing		
[mm]	%		
31,5	100,0		
22,4	86,0		
16	71,2		
11,2	57,4		
8	46,2		
4	40,5		
2	37,9		
1	36,2		
0,5	33,0		
0,25	27,5		
0,125	25,8		
0,063	25,3		
0,052	24,3		
0,0383	22,2		
0,0252	17,8		
0,0192	14,8		
0,014	13,5		
0,0098	12,8		
0,0074	12,5		
0,0051	12,3		

0,0034

12,0

Results		
D ₈₅	21,88	
D ₆₀	11,98	
D ₅₀	8,96	
D ₁₅	0,02	
D ₁₀		

Coefficient of uniformity			
D_{60}/D_{10}			



Made By: THAND

Checked By: CVZ

Date: 15. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11

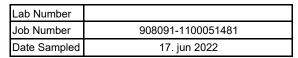


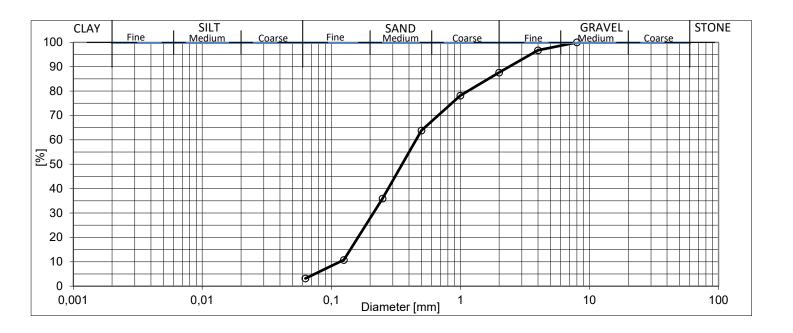
Inspection Section	BH05-S11-5
Sample Description	Sand, medium - coarse
Location	Gilbjerg Hoved

_	ŭ	
		sults
	D ₈₅	1,65
	D ₆₀	0,45
	D ₅₀	0,35
	D ₁₅	0,14
	D ₁₀	0,12

Sieving		
Sieve size	Passing	
[mm]	%	
8	100,0	
4	96,7	
2	87,6	
1	78,1	
0,5	63,8	
0,25	35,9	
0,125	10,8	
0,063	3,2	

Coefficient of uniformity		
D_{60}/D_{10}	3,90	





Made By: THAND

Date: 17. jun 2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



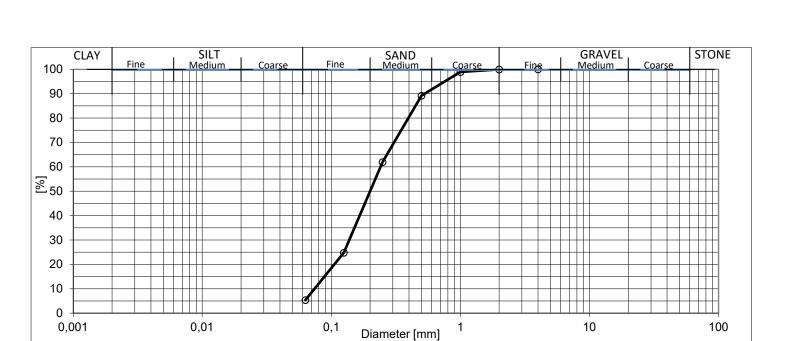
Inspection Section	BH05-S21-10
Sample Description	Sand, medium
Location	Gilbjerg Hoved

Sieving			Res	sults	
Sieve size	Passing		D ₈₅	0,45	
[mm]	%		D ₆₀	0,24	
4	100,0		D ₅₀	0,20	
2	99,9		D ₁₅	0,09	
1	98,9		D ₁₀	0,07	
0,5	89,2				
0,25	61,9		Coef	ficient of u	niformity
0,125	24,8		D_{60}/D_{10}	3	,25

5,4

0,063

Lab Number	
Job Number	908091-1100051481
Date Sampled	17. jun 2022
-	



Made By: __THAND

Date: 17. jun 2022

Checked By: CVZ

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



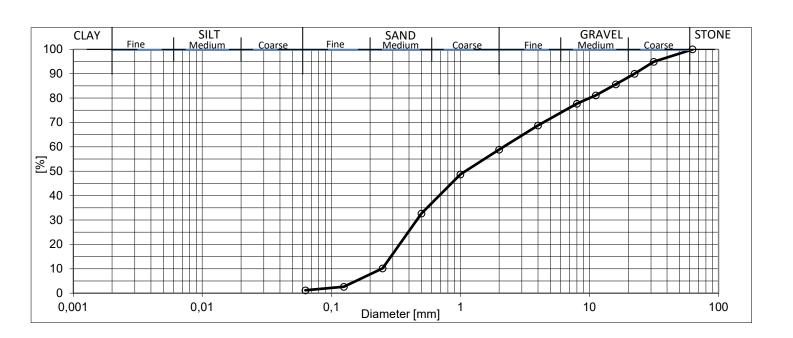
Inspection Section	BH05-S31-15
Sample Description	Sand, coarse
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	20. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
63	100,0
31,5	94,9
22,4	90,0
16	85,6
11,2	81,1
8	77,7
4	68,8
2	58,9
1	48,7
0,5	32,7
0,25	10,2
0,125	2,6
0,063	1,2

Results	
D ₈₅	15,24
D ₆₀	2,16
D ₅₀	1,09
D ₁₅	0,29
D ₁₀	0,25

Coefficient of uniformity		
D_{60}/D_{10}	8,80	



Made By: THAND

Checked By: CVZ

Date: 20. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



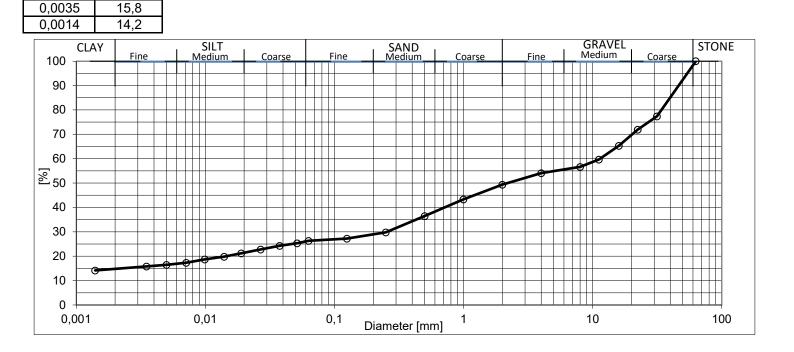
Inspection Section	BH05-S41-20	
Sample Description	Sand, medium - coarse	
Location	Gilbjerg Hoved	

Lab Number	
Job Number	908091-1100051481
Date Sampled	17. jun 2022

Location		
Sieving		
Sieve size	Passing	
[mm]	%	
63	100,0	
31,5	77,4	
22,4	71,9	
16	65,3	
11,2	59,7	
8	56,6	
4	54,1	
2	49,4	
1	43,3	
0,5	36,5	
0,25	29,8	
0,125	27,2	
0,063	26,3	
0,0514	25,3	
0,0377	24,3	
0,0268	22,8	
0,019	21,2	
0,014	19,8	
0,0099	18,7	
0,0071	17,3	
0,005	16,5	

Results	
D ₈₅	39,81
D ₆₀	11,44
D ₅₀	2,19
D ₁₅	0,03
D ₁₀	

Coefficient of uniformity		
D_{60}/D_{10}		



Made By: THME

Checked By: CVZ

Date: 17. jun 2022

TEST METHODS: DS/EN 933-1, DS/EN 933-9, DS/EN 1744-1, prVI 99-11



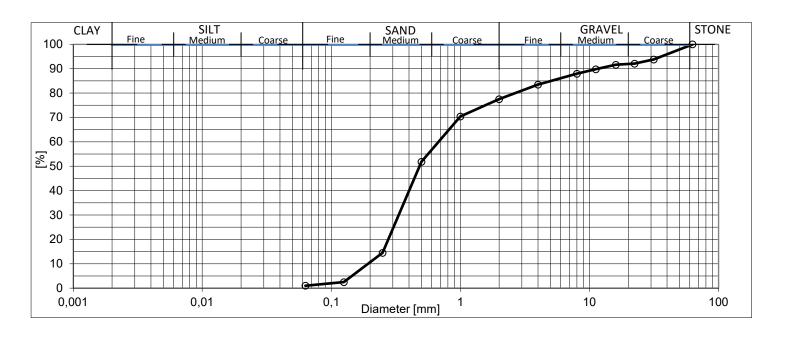
Inspection Section	BH05-S51-25
Sample Description	Sand, fine-medium
Location	Gilbjerg Hoved

Lab Number	
Job Number	908091-1100051481
Date Sampled	20. jun 2022

Sieving	
Sieve size	Passing
[mm]	%
63	100,0
31,5	93,8
22,4	92,1
16	91,6
11,2	89,7
8	87,9
4	83,5
2	77,5
1	70,4
0,5	51,8
0,25	14,5
0,125	2,5
0,063	1,0

Re	sults									
D ₈₅ 5,08										
D ₆₀	0,68									
D ₅₀	0,48									
D ₁₅	0,25									
D ₁₀	0,19									

Coef	ficient of uniformity
D_{60}/D_{10}	3,52



Made By: THAND

Checked By: CVZ

Date: 20. jun 2022

APPENDIX 6.1-6.5 - FIELD LOGS



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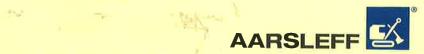
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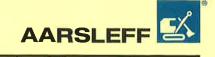
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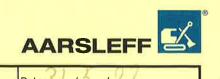
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Sag	g:2	32	1913-2	81	DGU-	nr:		Kote: TR	Inita S		Dato: 31 / 3 / 2 L									
Воі	ring:	13/	403		Lok.:	bil	lleleje	Bundpejl:	Vandsp	ejl:			Side:	5	a é					
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_	Pejlerør Prøver Ver terræn Dubde Prøve Prøve Lag-									Vingef		_	_	PT		Notater Dybde af: forgravning/	-			
OV	er terr	æn	Dybde m.	Prøve	Prøve- nr.:	Lag- grænse	Jordartsb	eskrivelse	Dybde m.	vinge	intakt kg	omrørt kg	dybde m	antal slag	Fore rør	udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH				
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over to	erræ	≘n	Dybde m.	Prøve	Prøve-	Lag- grænse	Jordartsbo	eskrivelse	Dybde m.	vinge	intakt kg	omrørt kg		antal slag	Fore rør	Dybde af: forgravning/ udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH			
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Sag:	32/	93	- 281	DG	U-nr	7.5					Kot	e: 🧲	M.		I	nit.: 🤵	5 -	Vic	-	Dato	:3//	51	8	
Boring:	13/	10	3	Lok	:: (<u>(</u>	Ti	118	lei	e e		Bur	ndpejl.;			V	andspej	l:			Side	: 1	a	Į.	
Foringsr	ør:	<i>[//</i> }	Ja 🔃 N					Į	3/1		Filte	erdim.;	25	NIMI.	S	lidsestr,	:			Sand	lstr.:	3		
	_	Prøve	er	Г											_		_							
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Bori	ng:	1	Hoy		Lok.:	67	letrie	Bundpejl: 3000	Vandspe	ejl:			Side:	_ t	a f	7
Fori	ngsr	ør:	K	Ja 🗌	Nej	Boring	gsdim.:	Filterdim.: 0 25	Slidsesti	r _e	8		Sands	str.: 7	121	BAGETYE
\vdash	ejlerø er terr	_		Prøve	Prøver	Lag-				Vingefo	intakt	omrørt		PT antal slag		Notater Dybde af: forgravning/
		ω.ii	Dybde m.	ripve	nra	grænse	Jordartsbe	eskrivelse	Dybde m.	Aurke	kg	kg		pr. 7,5 cm	Fore rør	udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH
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Vitarelin		-	80_						80_					55		
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Si	ag:7	32	193-	281	DGU-	nr		Kote:	Init.:	55.	NG		Dato:	/	/	
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Fo	orings	srør:	X	Ja 🗌	Nej	Borin	gsdim.; 811	Filterdim.: 0,25	Slidsest	r.:	6		Sands	tr.: 7	12-8	HEFTED
F	Pejler	cohr			Prøver					\				DT		Nista
H	over ter	_	0.1.1	Prøve	Prøve-	Lag-				Vingefo	intakt	omrørt		PT antal slag	_	Notater Dybde af: forgravning/
l]	Dybde m.		DE de	grænse	Jordartsb	eskrivelse	Dybde m.	Villac	kg	kg		pr. 7,5 cm	Fore rør	udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH
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a		0	900		19		Stonet		700				8,98	8 11		der
	X I		20_						20_							
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Sa	g:Z	32/	93-7	X)	DGU-	nr:		Kote:	Init.:	55	403		Dato:	91	51	22
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L	Pejler	ør			Prøver					Vingefo	orsøg		S	PT		Notater
01	ver ter	ræn	Dybde m.	Prøve	Prøve- nr ₊ :	Lag- grænse	Jordartsb	eskrivelse	Dybde m.:	vinge	intakt kg	omrørt kg	dybde m	antal slag	Fore rør	Dybde af: forgravning/ udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH
-		11	20				Sans, grus, Ston SV. lerer	ut, +Er, 145 Blan								
1		1	40				SV. lever	Ole	40							
1	2	/	60		26				60_							
		1	80						80_							
1		11	/300		27				300				13,0			Lukhat Sona.
-	3.	-	20_						20_					500		in Ban.
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-			60_		28	13.7			60_							
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ŀ		-	20_			WARD I	0		20_							
Ž		-	40_				THE REAL PROPERTY.		40_				143			Lukbat Sow
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Ä		1	80				V 1		80_							
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×		12	60_						60_							
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λ			20_						20							
	4	10	40_		36				40_				19,5			Lukket Soil
7		4	60_		2.6				60_	ļ				6 9		and Ram
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54.		×	00		37				/800					00		700



s	ag:	13	21	93-2	31	DGU-	nr:		Kote:	Init.; S	5-1	16		Dato:	10 1	51	22
В	orin	g:	, H	104		Lok.:	Gil	lelest	Bundpejl: 50,00	Vandspe	ejl:			Side:	4	a 9	\
F	oring	gsrø	r:	X.	Ja 🗌	Nej	Borin	gsdim.: 811	Filterdim.:0,25	Slidsest	rvi	ě		Sands	tr.: 7)	13A	GE TID
	Peil	erør				Prøver					Vingefo	orsøg		SI	РТ		Notater
H	over I		n	Dybde	Prøve	Prøve-	Lag-			Dybde	vinge	intakt	omrørt		antal slag	Fore	Dybde af: forgravning/ udtagne miljøprøver/
				m.		nr	grænse	Jordartsb	eskrivelse	m.		kg	kg	m	pr. 7,5 cm	rør	fræsning (fra/til)/ omrigning til DTH
,		-	101	20				Sand, SV. leter,	916, Stement	20							
	4	i.		40				175 Drun, tor.		40							
5				60		38				60							
	ĸ		2	80						80							
1				900		39				1900				19,0			Lukkel Soul
	-			20		2.1				20				-/-	186		ind Am 2 cm
	100		-	40						40	ATT COME			la a	32		MARGIET 9 CM
	E			60		40				60				1934	39	-	Tor
,			-	80				1		80							
l	-		9			1				00	· · · · · ·						
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	4		1	80_						80_							
			5	300		47				2300							
	-		28.70	20						20_							L.
	-		2	40_				*		40_		cienti ene	: 4(7)	23,5			Most bec
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N. Carlot	× ×		1	80						80		1			1213		
1	T _{al} i		-	2400		49				2900	T	1		23,95	212		Test

Sa	g: /	23	2143	-26	DGU-	nr:		Kote:	Init.: 5	S. L	6		Dato:	11 1	51	22	
Во	ring	: /	3404		Lok.:	Gill	eleje	Bundpejl: 60,00	Vandspe	ejl:			Side:	5	a	7	
Fo	ring	srør	: 4	Ja	Nej	Borin	gsdim.: 811	Filterdim.: 0,25	Slidsest	r _e j	*		Sands	tr.:7)	LBA	GE FILD	
			8														
\vdash	Pejle	_			Prøver					Vingefo				PT	_	Notater Dybde af: forgravning/	
°	ver te	erræn	Dybde	Prøve	Prøve- nr.:	Lag- grænse	Jordartsb	eskrivelse	Dybde m.	vinge	intakt	omrørt	dybde	antal slag	Fore rør	udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH	
L			20		Sva.		SANGSU. LON US. Brow. TOR	ry, gros, Stepa	20		kg	kg	m	or₌ 7,5 cm		Offinghing the Diff	
-			40				US. Brew. Tor		40				İ				
1.		1	60		50				60								
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-		7			-				2500				250			Libbet Son	L
	1		20		-51				20					50		ind Ra	
		1	20_						40					870		MANGE & C.	
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Vaucebjes		1	80_						80_					27		26	
1		1	3000		51				300				24/19	25		tall	



c-	93	7	93-2	Q I	DCH	n.e.		vara To	0	SI	1			10	5	77
Sag	-	1	HALL	01.	DGU		it E	Kote:	Init.:		16-		Dato:	7	2 /	LL
Bor	ing:	10	na Corp	_	Lok.	(11)	lletyre	Bundpejl: 50,000	Vandsp	ejl:			Side:	6	a	7
For	ings	rør:	X	Ja 🗌	Nej	Borin	gsdim.: 677	Filterdim.: 0, 25	Slidsest	r.:			Sands	str.:	12	BAGE TYL
-	ejler	ør			Prøvei					Vingefo	ors da			PT		Notater
\vdash	er teri		Dybde	Prøve	Prøve-	Lag-			Dybde	vinge	intakt	omrørt	_	antal slag		Dybde af: forgravning/
L			m.		nr.:	grænse	Jordartsbe	eskrivelse	m.		kg	kg	m	pr. 7,5 cm	Fore rør	udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH
-		×	20_				Sand, grus, sv. hero	+ Stenet, Mari	20_							
:4			40_		14	304	1300		40_							
r		ř	60		-62		Sand, grus, VF.,	Ston gra	60_							Salt Show
		-	80_						80_							V
1		4	⅓ 00_		-63			6	3100				31,6			LUKKet Somze
7		N.	20_						20_					205		INT BUT 60 EN
1		* 1	40_		64				40_				21.21	14		Vo. Dunie
۲.		-	60_		67	3/,7			60_				31,39	10		
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6		9	3700		65				₹00							
1			20_						20_							
100		0.1	40_		23				40_				775			LUNKET SOM
97. 4			60_		66				60_				J.A.D	22		Mounte
4		*	80_					1	80_					21		Monaler Com
-		1	³³ 00		67				00 🕑				•	21		10 17 mm
		1	20						20_					930		VS. ILIAMUT
3			40		M.				40_							
			60_						60_							
			80_						80_							
-		1	3400		69				700				349	ウト		LUMBER SOWS.
1		6 9	20_						20_					32		Monday 1156.
0 14		4	40_		70		1	. 1	40				34,24	24/11		VS. 15,5 MUT
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2.0			40_		72				40_				35,5			Lokket Souls
)	60_		IL				60_					214		Mc Can Mungley
			80_						80_					36		ranger 10,5cm
1		-	M no		37				Z/nn					20.00		V S 21 2 20 E



Sag:	321	43	1861	DGU-	nr:		Kote:	Init.:	5. h	6		Dato:	16 1	5 1	22
Boring:	31	404		Lok.:	Gil	lelee	Bundpejl: 50,00	Vandspe	ejl:			Side:	7	a 💪	7
Foringsrø	ør:	4	Ja 🔃	Nej	Borin	gsdim.: 6	Filterdim.: 0, 25	Slidsest	r.X	,		Sands	str.i /	1.8	9GE FYLD
Pejlerør		TVI I		Prøver	-				Vingefo	orsøg		s	PT		Notater
over terræ	-	Dybde	Prøve	Prøve-	Lag-			Dybde	vinge	intakt	omrørt	_	antal slag	_	Dybde af: forgravning/ udtagne miljøprøver/
		m.		nr	grænse	Jordartsb	eskrivelse 	m.		kg	kg	m	pr. 7,5 cm	Fore rør	fræsning (fra/til)/ omrigning til DTH
5	-	20_				Sand, Stus, St. S	itement VF. 972	20_							
5	i den	40_						40_							
		60_		74	36,7			60_							
4	4	80			5041	> St. Stenet + S	t. gruset	80_							
<u></u>	1 3	00		25				\$9oo				37 ₀			Luldiat Sonbe
1	-	20						20_					12		ind DA-
21 20	,	40_		20				40_	ļ		ļ	37-3	34		VS. 18,3 MUZ
3	8	60_		76				60_							
7	i	80			348			80_							
600	3	<u>%00</u>		79		Stonet,		3800							
(A)	Ŧ	20						20_							
À	5 -	40		0.47				40_				385			Lutytet Soul
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	4	80						80_					78		
	7 3	00		79				3900				39,95	09		VS 196 mut
-	V-1	20						20_							
3	7	40_				1 '		40_							
7		60_		80				60_							
4	3	80_						80_		ļ					
	¥	00		81				00	ļ	ļ		40,5	470.0		Lukkel SON
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Υ.		60_	-	82				60_				10,0	- t		VS 11/0/MgT
	数 []	80			408	Mem SANDET		80_		ļ	ļ				-
7	1	00		83		THERE STANDET		4100	ļ						
3		20			9/30			20							
ě.	SANDIGIA, SU						Grebat, VF.	40_	ļ			411.5			Luniet Sonle
3	4	60_		.89			,	60_	ļ			247.2	68		Jee Roy-
	1	80_											88		
7	: 4	200		85				1/200					10		VS. 12,9mm



Sa	g:2	3	219	3-28	DGU-	nr:		Kote:	Init.: 5	5.	16		Dato:	17,	51	22
Во	ring:	B	404		Lok,:	61	lebere	Bundpejl: 50,00	Vandspe	ejl:	1		Side:	8	a Z	7
Foi	ings	rør:	3	Ja 🗌	Nej	Borin	gsdim.;	Filterdim.: 0, Z5	Slidsest	r.:	ě		Sands	tr.: 7	上西户	IGE TYLD
Е	_			16												
H	Pejler				Prøver	i)				Vingefo	orsøg		S	PT		Notater
0/	er terr	æn	Dybde m.	Prøve	Prøve- nr.:	Lag- grænse	Jordartsb	eskrivelse	Dybde m.	vinge	intakt	omrørt	dybde	antal slag	Fore rør	Dybde af: forgravning/ udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH
		1	20				Sand, grasgr	uset VF	20		kg	kg	m	ior. 7,5 cm		offinghing the DTH
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1		1	20_		107				20_					1011		ING TRAM
6		1	40_		-				40_				43,48	108		VS. 17.8mt.
1		10 St	60_		.88				60_							
4		Į.	80_				1		80_							
8		3	00		-89				00							
,		1	20_					1-	20_							
- P		×.	40_		n.				40_				445			Lukset Sonda
1		*	60_	£1	.90				60_					10 5		und Ram
1			80_						80_					88		
1		1	900		91				9900				44,99	38		VS. 19.7 mg
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1		Å.	80_						80_							
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1		ř.	20_						20_					0		IND RAM
9		5	40_		Chris				40_				46,45	7 8 3 4		Vs 24,10
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9 A		8	00		-95				00			10.000.00				
1.	3	3.1	20_				*		20_				43.4	5		
1		A.	40_		96				40_				機動	16 -		LUKKET SOME
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			80_		0~				80_				4490	26		V5 27,00
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S	ag:ŋ	31	193		DGU-	nr:		Kote: TR	Init.:	PKN	Jun	YSE	Dato:	23/	51	22
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H	oring	-6.13	100	-				Filterdim.: 0,25	Slidsest	ra	Ē		Sands	str. S	AND	HG FILTER
ŀ	Pejle	rør	53		Prøver					Vingefo	orsøg		S	РТ		Notater
Ī	over te	rræn	Dybde m.	Prøve	Prøve- nr-:	Lag- grænse	Jordartsbo	eskrivelse	Dybde m.	vinge	intakt kg	omrørt kg	dybde m	antal slag	rør	Dybde af: forgravning/ udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH
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١			40 <u></u>	2 2					40_ 60_							
١			80_						80_							
			20_						20_							
			40_				ii		40_							
			⁶⁰ _						80_							
			00						00							
			²⁰ –				×		²⁰ _							
(4)			60_						60_							
1			80_ 00						80_ 00							



Sag	Š.	1	13,-1	7.5	DGU-	nr:		Kote:	Init.:	13	- 64	t ,	Dato:	251	41	22
Bor	ing:	F	9.6		Lok.:	n.	1 - in	Bundpejl:	Vandspe	ejl:		2,75	Side:	1	a	
Fori	ngsı	rør:	1,7	Ja 🗌	Nej	Borin	gsdim.:	Filterdim.:	Slidsest	r.t			Sands	str.:		
									_							
P	ejlerø	ør	1		Prøver					Vingefo	orsøg		S	PT		Notater
ove	r terr	æn	Dybde m.	Prøve	Prøve-	Lag- grænse	Jordartsbe	eskrivelse	Dybde	vinge		omrørt		antal slag	Fore rør	Dybde af: forgravning/ udtagne miljøprøver/ fræsning (fra/til)/
			20				CARER SUD	1818.1	20		kg	kg	m	pr. 7,5 cm		omrigning til DTH
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			60_						60_							
			80						80							
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			60_				T- m:		60_							
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Fo	rings	srør	▽.	Ja 🗌	Nej	Borin	gsdim.:	Filterdim.:		Slidsest	r.:			Sands	str.:			
E																		
	Pejler	ør			Prøvei	r					Vingefo	orsøg		S	PT		Notater	
01	er ter	ræn	Dybde m.	Prøve	Prøve-	Lag- grænse	Jordartsbo	eskrivelse		Dybde m.	vinge	intakt	omrørt		antal slag	Fore rør	Dybde af: forgravning/ udtagne miljøprøver/ fræsning (fra/til)/	
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Sag: Boring: Foringsrør: Ja					DGU-	nr;		Kote:	Init.	14	Q'B		Dato:	251	4	22	7
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For	ings	rør:	<u></u>	Ja 🗌	Nej	Borin	gsdim.: ///	Filterdim.	Slidsest	ra	A		Sands	str.:			
	ejler	dr			Prøver										_		1
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			Dybde m.		nr	grænse	Jordartsbe	eskrivelse	Dybde m.	vinge	intakt kg	omrørt kg	dybde	antal slag	Fore rør	udtagne miljøprøver, fræsning (fra/til)/ omrigning til DTH	"
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Sag: 7 (95-78) DGU-ni							GU-nr:		roen	Init.: VA PB			Dato: 75/ 1/22					
Boring: BHo5 Lok.: Callelere								Bundpejl:		Vandspe	Vandspejl: Side: [4] a							
Foringsrør: Ja Nej Boringsdim.:				Filterdim.:		Slidsestr.: Sandstr,:												
																		1
	Pejler	ØГ			Prøver						Vingeforsøg			SPT			Notater	
over terræn		Dybde m.	Prøve	Prøve-	Lag- grænse	Jordartsb	eskrivelse		Dybde m-	vinge	intakt kg	omrørt kg		antal slag	Fore rør	Dybde af: forgravning/ udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH		
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Sa	9:2	521	93-29	16	DGU-nr:			Kote:	Init.: 14-88				Dato: 25/4/22			
								Bundpejl:	Vandspe	ejl:	é		Side: 5 a 5			
						Filterdim.;	Slidsest	r.:			Sandstr.:					
Pejlerør			Prøver				Vingeforsøg		SPT		Notater					
over terræn		Dybde m.	Prøve	Prøve- nr.t	Lag- grænse	Jordartsbe	eskrivelse	krivelse Dybde m.		intakt kg	omrørt kg		antal slag	Fore rør	Dybde af: forgravning/ udtagne miljøprøver/ fræsning (fra/til)/ omrigning til DTH	
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APPENDIX 7.1 - RISK LOG

Energinet-Gildbjerg Hoved HDD

Risk Log

2022-06-30

Description

Risk level

1 Risk description:

Boulders in soil - large

Cause description:

Cause #1: Primarily boulders in clay till, gravel and dense gravel in the glacier layer.

Cause #15: Several boulders resulted in delays of Geotechnical boreholes.

Cause #19: Encountering one or several large boulders during HDD can cause delay in operations.

Effect description:

Effect #1: Can lead to withdrawal and new HDD pilot hole.

Control-4:

Minimize drilling distance in clay till and gravel layers.

Possible - No deadline

12 Risk description:

Not possible to maintain sufficient pressure

Cause description:

Cause #12: Relevant for sand deposits. In dry sand drilling fluid can flow into the soil. Drilling is expected in the late glacial and glacial sand layer.

Effect description:

Effect #10: New start up can be required or change of drilling procedure.

Control-11:

Minimize drilling in dry sand or coarse material.

Possible - No deadline

Control-12:

Design of drilling mud according to conditions - saturated and non-saturated.

Possible - No deadline

14 Risk description:

Drilling below and above water table

Cause description:

Cause #14: When there is a change going from saturated to unsaturated conditions this can cause loss of pressure.

Effect description: Effect #12: Can require more pressure and require adaptation of set-

Control-12:

Design of drilling mud according to conditions - saturated and non-saturated.

Possible - No deadline

8 Risk description:

Challenging drilling conditions and area constraints due to multiple parallel HDD

Cause description:

Cause #8: Blow out can lead to a situation with no feasible drilling route. Challenging due to long length of app. 1 km and sand deposits.

Cause #16: Up to three parallel HDD routes. Effect description:

Effect #7: In worst case new alignment or new contractor is required.

Time



Time



Time



Time



16 Risk description:

Boulders in soil - small

Cause description:

Cause #1: Primarily boulders in clay till, gravel and dense gravel in the glacier layer.

Effect description:

Effect #14: Withdraw and minor rerouting of HDD

2 Risk description:

Limited geotechnical information between Gilbjerg Hoved HDD and offshore cable survey

Cause description:

Cause #2: Uncertain geology towards sea at marine exit point. Nearest geotechnical borehole is on 10m water depth.

Effect description:

Effect #13: Uncertain to contractor how plan the work at sea.

Control-6:

Contractor shall familiarize himself with the cable route survey

reports.

Possible - No deadline

5 Risk description:

Blowout, damage to road

Cause description:

Cause #5: HDD crossing of road at Gildbjerg Hoved. Start point is close to surface of the road.

Effect description:

Effect #4: Settlements of road leading to potential closure of road.

Control-7:

Safe distance from HDD starting point to road and amount of overburden between HDD and road.

Possible - No deadline

6 Risk description:

Blowout, damage to surroundings

Cause description:

Cause #6: Gildbjerg Hoved is nature 2000 area. Probability dependent of depth of drilling which is expected in the post glacial layer.

Effect description:

Effect #5: Blow out can results in drilling mud at surface in Natura 2000 area, this can cause a delay require cleaning up and informing authorities.

Control-2:

Response plan shall handle this to minimize impact on area.

Possible - No deadline

Control-8:

Increase overburden.

Possible - No deadline

Control-9:

Manage use of drilling mud additives

Possible - No deadline

4 Risk description:

Blowout, easy clean up

Cause description:

Cause #4: Probability is highest when drilling through sand deposits.

Effect description:

Effect #3: New drilling is required

Time



Time



Time



Time



Time



17 Risk description:

Large boulder reduces diameter of HDD casing

Cause description:

Cause #18: Encountering boulders when pulling pilot pipe may cause damage to pipe.

Effect description:

Effect #15: Damage to pilot pipe and increased friction and risk when pulling cable through pipe.

Control-15:

Design pilot pipe according to high voltage cable and HDD

according to pilot pipe.

Possible - No deadline

11 Risk description:

Risk of friction during drilling

Cause description:

Cause #11: Probability is in general largest in clay till.

Effect description:

Effect #9: Can lead to new reaming up several times leading to delays.

Pipes can get stocked due to friction.

Control-10:

Additives to drilling mud.

Possible - No deadline

15 Risk description:

Crossing of high voltage cable.

Cause description:

Cause #17: Depth of high voltage cable is not known.

Control-13:

Awareness of cable when planning HDD route.

Possible - No deadline

Control-14:

LER investigation and contact to cable owner

Possible - No deadline

10 Risk description:

Interference with roots

Cause description:

Cause #10: There can be roots from vegetation.

Time



Time



Time



Time

